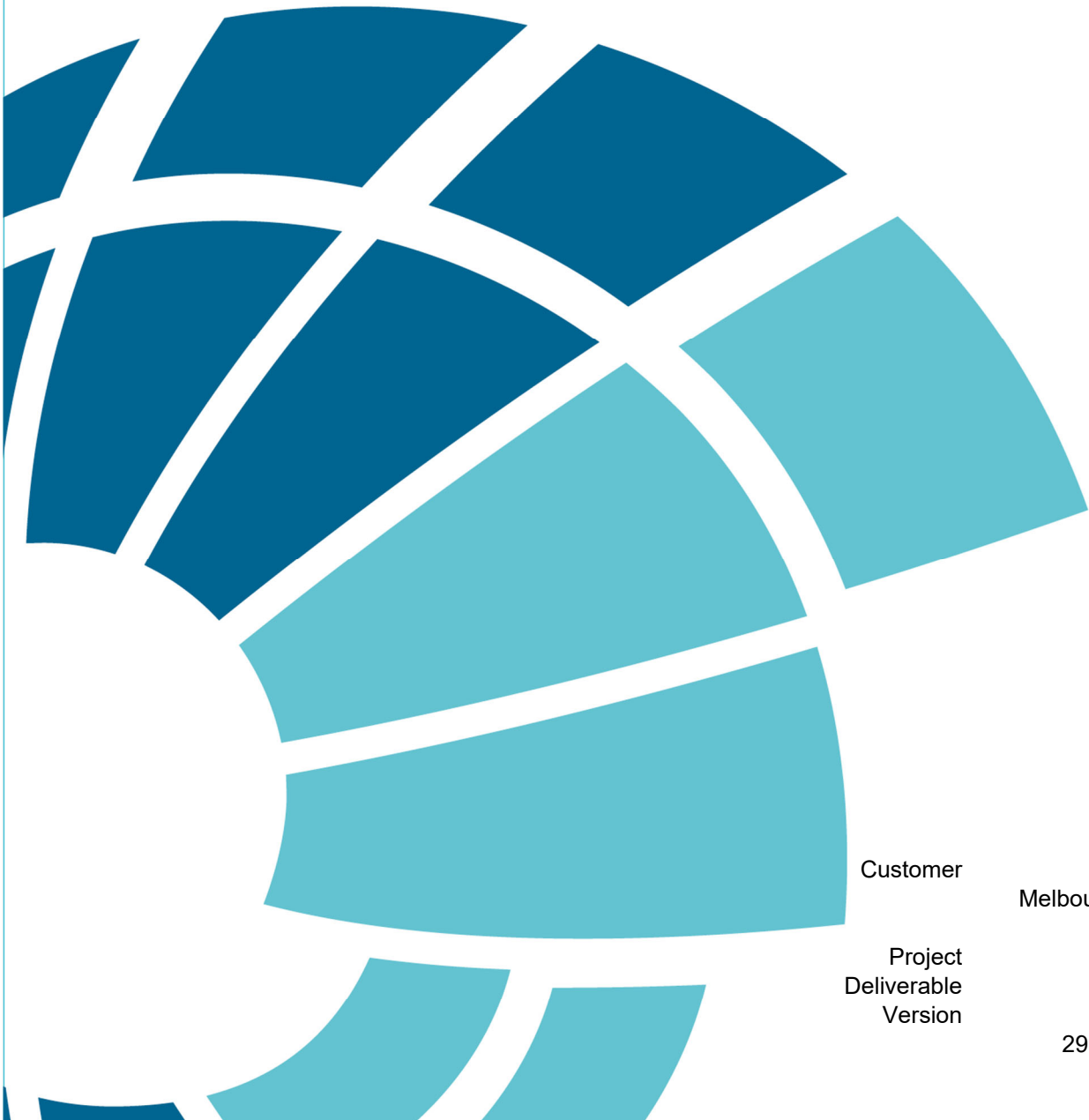


Coastal Processes and Marine Ecology Impact Assessment

Point Nepean Research and Education Field Station



Customer

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Author	Taylor Rubinstein, Luke Johnston
Reviewed By	Dr Daniel Machado, Jeremy Visser
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Amendment Record

The Amendment Record below records the history and issue status of this document.

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1 Introduction

The University of Melbourne (UoM) and Monash University are collaboratively working to the Point Nepean Research and Education Field Station (PNREFS). The facility is to be situated within and adjacent to Badcoe Hall at the Point Nepean Quarantine Station precinct in the Point Nepean National Park, Victoria.

As part of this project, UoM and Monash University have contracted BMT to design the seawater intake and outfall system which will feed the external tanks, laboratory and aquaria at the proposed facility. BMT has also been tasked with supporting the applications for approvals associated with marine aspects of the project. The proposed concept design of the seawater intake and outfall system is outlined in the following section of this report. Based on the proposed concept and regulating agencies advice (DELWP, Parks Victoria, EPA), the marine approvals required for the project are:

- Consent under the *Marine and Coastal Act 2018* ('MaCA Consent').
- Development Licence for prescribed development activity B03 – Fish farms under the *Environment Protection Act 2017* (EP Act) and regulations ('EPA Development Licence') OR a licence exemption.
- Operating Licence for prescribed development activity under the EP Act and regulations ('EPA Operating Licence') OR a licence exemption.
- Works Permit in a Waterway / Marine Area – to be sought before construction from the Local Port Authority (Parks Victoria for the location of this project).

Each of these approvals require information regarding the potential impacts of the coastal and marine construction works and the facility while in operation on the marine environment.

In line with the above, and in support of applications for all approvals, this assessment will consider the impacts of the proposed works on the marine and coastal environment, with focus on following:

- Coastal processes (including waves, tides and sediment movement)
- Marine ecology in the study area
- Water quality

The objectives of this report are to support applications for the various consents and licenses mentioned above. Initially, this report will also support the submission of an Environmental Effects Statement (EES) referral to the Department of Land Water and Planning (DELWP) by the project team.

1.1 Study Area

The Point Nepean Quarantine Station Precinct (see Figure 1.1) is located on the northern coast (Port Phillip Bay side) of the Mornington Peninsula approximately 4km east of the tip of Point Nepean. The precinct and the broader Point Nepean National Park area are culturally important locations for peoples of both indigenous and European descent. Pre-colonisation, it is reported the area was predominantly utilised by women with areas for birthing, women's ceremony, and initiation rituals (Bunurong Land Council Aboriginal Cooperation, n/d).

After colonisation, the area became the 'Quarantine Station' where boats travelling to Melbourne would anchor offshore and be quarantined for a time to ensure no viruses or sicknesses would be transmitted to local Melbournians when passengers disembarked. In some cases, many people died while in quarantine and were buried in cemeteries within what is now Point Nepean National Park. The army

assumed control of the facilities from 1950-1998 and introduced the Officer Cadet School. After this, the area became the Point Nepean National Park (Nepean Historical Society, n/d).

Offshore, the study area is predominantly characterised by strong tidal currents which run through Port Phillip Heads. Ocean swells propagate through the Heads in some instances and impact beaches at the Quarantine Station, however the area is largely protected. There is a seawall at the back of the beach in front of the Parade Ground and more naturalised dune systems west of this. During summer months, the Point Nepean National Park is popular predominantly for swimmers and bushwalkers.



Figure 1.1 Study Area Map – PNREFS

2 Planned Works

Designs for the marine components of the proposed seawater system for the PNREFS have been developed by BMT (up to Concept Design level). The design includes the following:

- Horizontal directional drilling (HDD) of the seawater intake and outfall pipes from the PNREFS building next to Badcoe Hall to a location approximately 170m offshore (as indicatively shown on Figure 2.1).
- A seawater intake valve inside a precast concrete end-structure protected from entrainment and intake of biota by a mesh screen. This will be sitting on a concrete mattress to protect from scour.
- A seawater outfall system with a diffuser nozzle outlet to be protected by a concrete structure to shield from falling objects (e.g., anchors). This will be sitting on a concrete mattress to protect from scour.

Installation works will include:

- Excavating a pit inland from which to begin the HDD operations (which will follow the technical guide attached in Annex B).
- Drilling the pipelines from the landside and pushing the pipelines through. This step will use natural drilling lubrication fluids such as a bentonite (clay) slurry.
- Small scale excavation of the seabed (using air lifting of materials) to reach the seaward end of the HDD pipe.
- Concrete encasing structures around both the intake and outfall will be pre-cast and lowered into place during construction.

Once installed, effluent emitted from the outfall is modelled to disperse fully, to background seawater concentration levels, within 25m from the outlet diffuser.

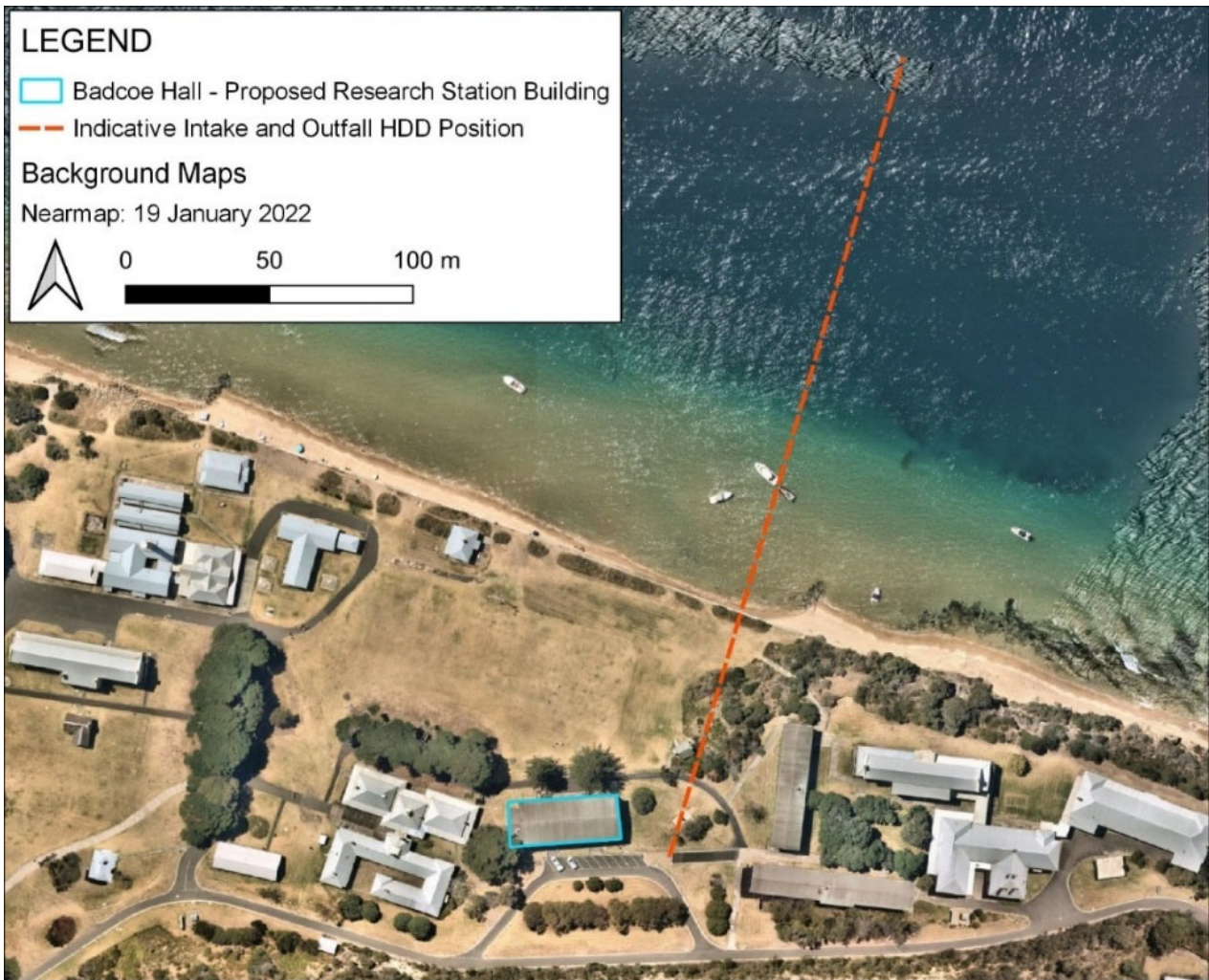


Figure 2.1 Proposed marine components of the Seawater System for the PNREFS

2.2 Policy Context

The project team, including Urbis as planning consultants, have previously completed an in-depth review of all potentially relevant legislation to identify all approvals that UoM and Monash University will require for the development of the PNREFS. Key approvals/studies required include an EPBC Self-Assessment, Heritage Permit, Cultural Heritage Management Plan, Planning Permit/Approval, MACA Consent, EPA Development License, EPA Operating License, Works Permits.

BMT are currently supporting UoM and Monash University to prepare the MACA Consent application, as well as the EPA approvals, which pending on feedback from the EPA may take the form of either licenses or license exemptions. The policy context for each of these approvals are outlined below. We note that additional approvals to these are also required, however the focus of this report is on the marine components, as outlined above.

Marine and Coastal Act (2018) Consent

For consent to be granted under the MACA the proposed works must meet certain criteria as follows.

The proposed works must be in line with:

- The objectives and guiding principles set out in Part 2 of the MACA.

- The Victorian Marine and Coastal Strategy 2014
- Any plans prepared under a regional and strategic partnership which may apply to the land.
- Any applicable environmental management plan
- Any applicable coastal or marine management plan
- Any relevant coastal recommendation

More specifically, the MACA consent assesses the long- and short-term Environmental, Social and Economic impacts of the proposed works. This includes impacts to coastal processes and the marine and terrestrial coastal environments, including impacts to individual at-risk species and coastal communities.

EPA Development License

For a development license to be granted by the EPA, the proposed works must meet the provisions set out under s69(3) of the *Environment Protection Act (2017)* as follows.

The proposed works must:

- Comply with the general environmental duty
- Have acceptably low potential impacts to human health, and the environment, including the impact on any environmental values identified in any relevant environment reference standard
- Comply with the principles of environment protection
- Utilise the best available techniques or technologies
- Comply with the EP Act and relevant regulations
- Also be assessed favourably by any other required regulatory agency

Specifically, the EPA Development approval will assess the potential impacts of the proposed works on human health and the environment regarding contamination and/or pollution of air, noise, water land and groundwater, odour and waste. For the PNREFS, impacts such as these may stem from the ocean outfall and the laboratory solid waste stream.

EPA Operating License

For an operating license to be granted by the EPA, the proposed works must meet the provisions set out under s74(3) of the *Environment Protection Act (2017)*. These provisions are the same as those listed above required for the Development License. The Operating License is the mechanism whereby the EPA prescribes any required conditions which must be met during operation of the facility. These conditions will likely include limits to concentrations of certain potential pollutants in the effluent water to ensure minimal impact to water quality in the local area from the facility.

EPA License Exemption

For an exemption for either a development license or an operating license to be granted by the EPA, the proposed works must meet the provisions set out under r24 of the *Environment Protection Regulations (2021)* as follows.

The proposed works must:

- Comply with the purposes of the EP Act
- Comply with the Objective of the Authority

- minimise litter and waste disposal by encouraging the management of waste in accordance with the EPA waste management hierarchy
- promote waste reduction, resource recovery and resource efficiency
- minimise the impact on human health and the environment from waste generation and waste disposal
- Comply with the principles of environmental protection
- Not adversely affect human health or the environment
- Not adversely affect the interests of any person other than the applicant
- Not adversely affect any environmental values identified in any relevant environment reference standard
- Utilise the best available techniques or technologies for engaging in the prescribed activity
- Have previously adequately engaged with any person whose interests may be affected by the proposed exemption

Further to this, the applicant must:

- be a fit and proper person
- have supplied sufficient further information to the Authority within a reasonable time if requested by the Authority.

The EPA must also consider whether it is in the public interest to grant a license exemption.

The application for a license exemption if appropriate, still requires the applicant to supply an acceptable level of information to the Authority concerning the potential impacts of the proposed works on human health and the environment from contamination and/or pollution of air, noise, water land and groundwater, odour and waste, with a focus on the ocean outfall and the laboratory waste streams.

3 Methods

3.1 Coastal Process Impact Assessment

The Coastal Process Impact Assessment consists of two key assessments:

1. Desktop analysis of previous studies

Previous studies relating to the Point Nepean Quarantine Station area supplied by UoM were reviewed. Some of these papers related to the Portsea front beach erosion and channel deepening project, by Port of Melbourne. We also reviewed publicly available online resources relating to coastal processes. These included:

- The Coast of Victoria: The Shaping of Scenery – (Bird, The Coast of Victoria, 1993)
- Report 04 Port Phillip Bay Wave Climate – (Cardno, 2017)
- Changes on the Coastline of Port Phillip Bay – (Bird, 2011)
- Hydrodynamics: Infrastructure Victoria Second Container Port Advice – (Cardno, 2017)
- Beach Report and Yarra Watch Results 2020-2021 – (EPA Victoria, 2021)
- Review of OEM assessment of potential causes of beach erosion at Portsea – CSIRO 2013 (Symonds & McInnes, 2013)
- Portsea Front Beach: Wave Modelling and Monitoring Investigation – (Advisian, 2016)
- Vic Tides 2022 – (Ports Victoria, 2022)
- Hydrodynamic Climate of Port Phillip Bay - (Tran, Provis, & Babanin, 2021)

Information contained in these reports allowed us to identify coastal process presently occurring at the Point Nepean Quarantine Station study site.

2. Historical Aerial Imagery and Bathymetry Survey Analysis

Bathymetric survey data collected in approximately 2010 was initially compared to historic aerial photography to determine the general form of the offshore bathymetry and to identify the location of the crest of an offshore sand slope. After this, twenty-eight historical aerial photographs were analysed for years between 1950 – 2022 to map the approximate location of the top of this sand slope. This allowed us to identify the likely zone of natural horizontal (toward and away from the shoreline) movement for the offshore sand slope.

3.2 Marine Ecology Impact Assessment

The marine ecology impact assessment included a desktop analysis of the following relevant databases to identify the key marine environmental values for the area (e.g., threatened, migratory or otherwise protected flora and fauna) and determine the likelihood of these values occurring in the study area:

- *Environment Protection and Biodiversity Conservation Act 1999* (Cth) (EPBC Act) Protected Matters Search Tool
- *Flora and Fauna Guarantee Act 1988* (Vic) (FFG Act) Threatened List (DELWP, 2021)
- *Fisheries Act 1995* (Vic) Protected Aquatic Biota List (Fisheries Victoria, 2021)
- Victorian Biodiversity Atlas (VBA) Survey Records (DELWP, 2022)

- Seemap Australia - Port Phillip Bay Marine Biotopes Dataset (Seemap Australia, 2019),
- Ramsar wetland Ecological Character Description for the 'Port Phillip Bay (Western Shorelines) and Bellarine Peninsula' Ramsar site (Hale, 2020).

The key marine environmental values likely to occur were then each considered in the context of potential impacts resulting from the construction and operation of the proposed PNREFS.

4 Coastal Process Impact Assessment

4.1 Geomorphology

The geomorphology of Point Nepean is defined by the presence (or absence) of dune calcarenite at the shoreline. This rock is formed as rain percolates through calcareous relic sand dunes over thousands of years, washing calcium from the upper, more recently deposited sand into horizons of cemented calcrete (or calcarenite) at depth beneath the dunes. When rising sea levels act to erode these relic dune systems, the hard calcarenite layers inhibit erosion of the otherwise sandy vegetated dunes.

This calcarenite can be seen prolifically in cliffs along much of the ocean coast of Point Nepean, as well as in headlands and reef platforms on the northern Port Phillip Bay coast such as at Fort Pearce and Police Point (Figure 4.1). Erosion of areas where the erosion resistant calcarenite is present tends to form cliffs and bluffs (Bird, The Coast of Victoria, 1993).

Where this calcarenite is not present at the shoreline, the backshore area is mostly flat and has little relief from the beach level. An example area with this geomorphology is the quarantine station and surrounding lawns (Figure 4.1). These regions are described as prograded Holocene dunes which formed at times of lower sea level shoreward of a former sea cliff in the calcarenite dunes. The extent of this former sea cliff can be seen clearly by comparing the landform elevation of different section of Point Nepean (Figure 4.1) (Bird, 1993).

Erosion in these areas has historically been an issue which has led to the construction of various coastal protection structures such as seawalls and groynes.

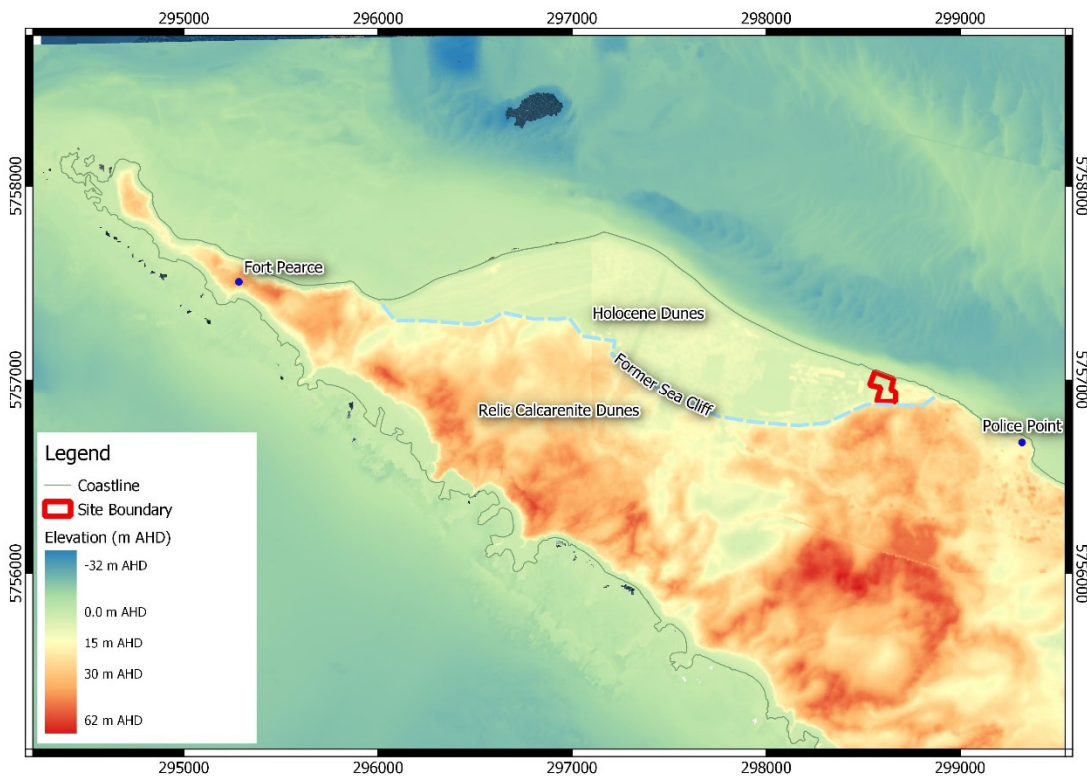


Figure 4.1 Point Nepean Geomorphology Map

4.2 Wind

Predominant winds at the site vary seasonally with predominant southerly – southwesterly winds in summer and predominant northerly – northwesterly winds in winter (Tran, Provis, & Babanin, 2021).

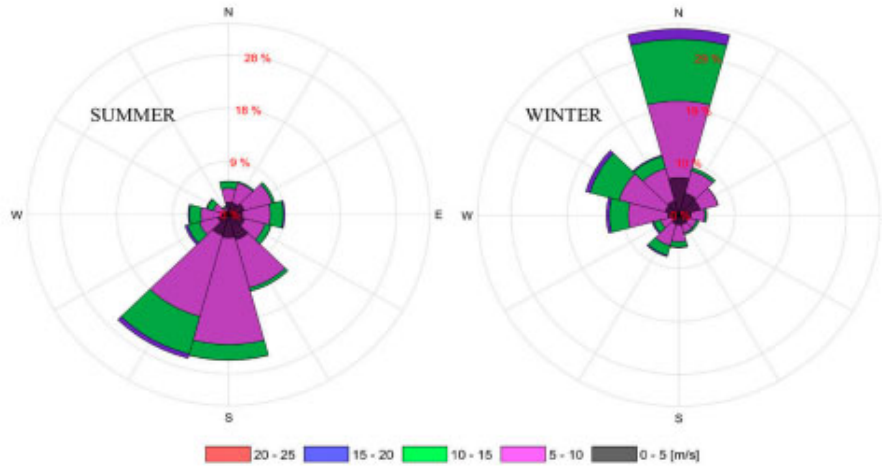


Figure 4.2 Wind rose 1998-2013 from South Channel Island station during summer and winter months (Tran, Provis, & Babanin, 2021).

4.3 Waves

Both long period swell waves and short period wind waves impact the site with varying magnitudes. These different wave sources are addressed individually below.

Swell Waves

Swell waves at the study site have the following characteristics:

- Waves propagate through Port Phillip Heads and refract around Point Nepean
- Due to local bathymetry, swell waves would be of minimal wave height by the time they refract to the shores immediately shoreward of the study site (Figure 4.3).
- Waves propagate differently through the heads under different tidal flows. Greatest propagation occurs at peak flood tide, moderate propagation at slack tide, low propagation at ebb tide (Cardno, 2017)

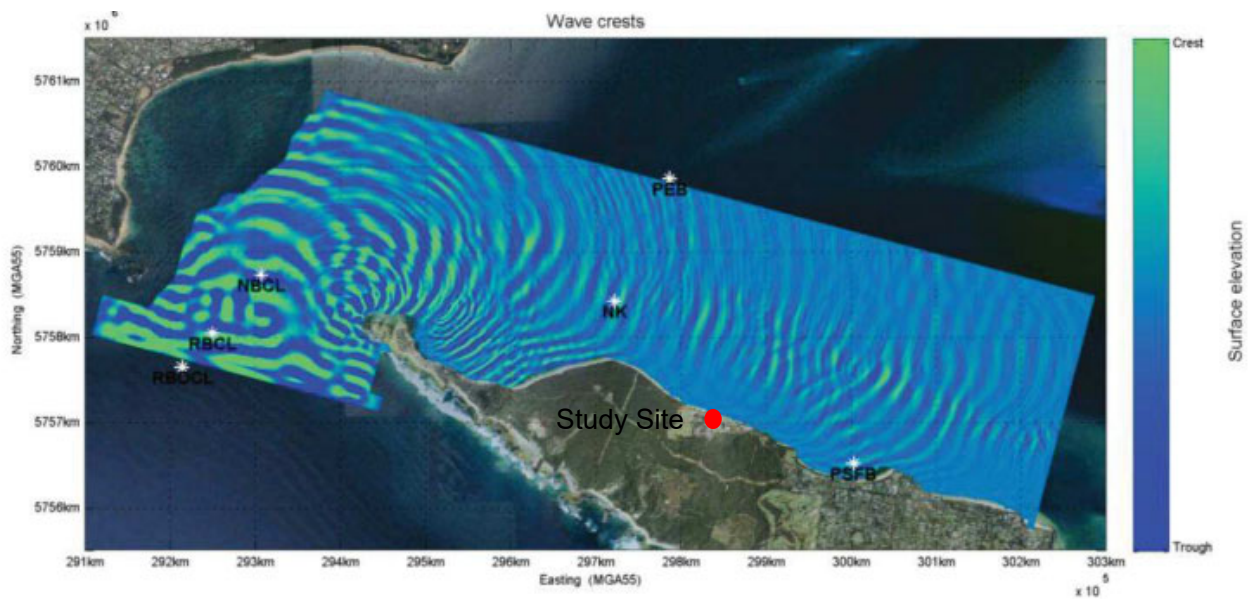


Figure 4.3 Modelled wave propagation and refraction into Port Phillip Bay (T = 20 swell waves) (Advisian, 2016).

Wind Waves

Wind waves impact the site when winds blow from the north through northwest (Figure 4.4), the prevailing condition during winter (Figure 4.2). The Point Nepean bay side shoreline (i.e, Ticonderoga Bay) is shielded from most of this wave energy by the Great Sands and Mud Island which sit immediately north of the study site (Bird, The Coast of Victoria, 1993). Despite this sheltering, wave heights can reach up to approximately 2 m at the study site under 1% AEP northerly winds (Cardno, 2017) (Figure 4.5).

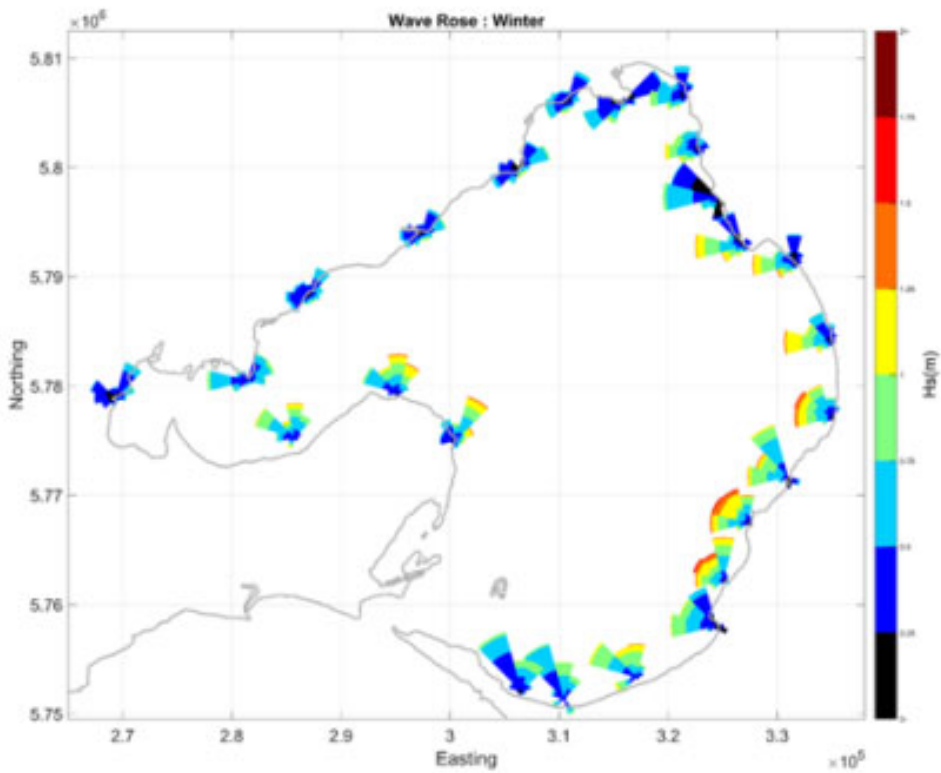


Figure 4.4 Winter wave roses for various locations around Port Phillip Bay including Blairgowrie (fair representation of Point Nepean) (Cardno, 2017)

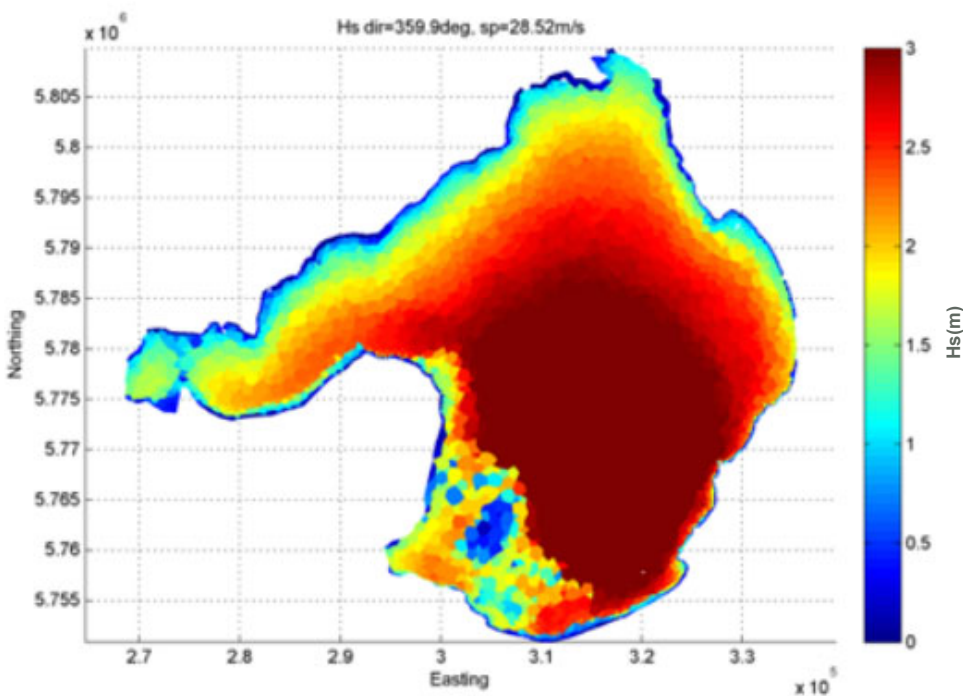


Figure 4.5 Wave heights associated with a 1% AEP northerly wind event (Cardno, 2017).

4.4 Tides/Currents

The tides at Port Phillip Heads are predominantly semidiurnal. The tidal planes for Point Lonsdale published by the Victorian Regional Channels Authority (VRCA) are provided in Table 4.1. Due to the attenuation of the tide within Port Phillip Bay, the tidal planes at the study site will vary to those presented in Table 4.1. VRCA (2021) suggest a 40% reduction in peak tidal water levels between Point Lonsdale and Portsea Pier. Prolonged data collection and analysis would be required to determine the actual reduction in peak tidal water levels from the Point Lonsdale tidal plane at the study Site.

Table 4.1 Point Lonsdale Tidal Planes (Ports Victoria, 2022)

AHD metres	Predominantly semidiurnal tides	Point Lonsdale Chart Datum metres
1.21	Highest recorded tide 12/07/1964	2.16
0.95	Highest Astronomical Tide (HAT)	1.90
0.62	Mean High Water Springs (MHWS)	1.57
0.36	Mean High Water Neaps (MHWN)	1.31
0.00	Australian Height Datum	0.95
-0.27	Mean Low Water Neaps (MLWN)	0.68
-0.53	Mean Low Water Springs (MLWS)	0.42
-0.95	Chart & Prediction Datum	0.00
-1.35	Lowest recorded tide 27/10/1972	-0.4

Current and future climate extreme water levels have been modelled along the Victorian coastline by McInnes et al. (2008). Storm tide values for Sorrento have been adopted for Point Nepean in this study (Table 4.2).

Table 4.2 Adopted current and future climate storm tide levels for Point Nepean (McInnes, Macadam, & O’Grady, 2008).

Annual Recurrence Interval (ARI)	Storm Tide Water Level Elevation (m AHD)		
	Current Climate	2030	2070
10	0.91	1.12	1.54
20	0.97	1.17	1.60
50	1.03	1.24	1.67
100	1.07	1.28	1.70

As tides fluctuate in Bass Strait, water moves in and out of Port Phillip Heads at varying flow rates. This occurs in the following way:

- Low tide in Bass Strait causes the greatest flow velocity from Port Phillip Bay out into Bass Strait. This can be up to 2 m/s at the heads and up to approximately 0.7 m/s in a North westerly direction at the study site (Figure 4.6).
- High Tide in Bass Strait causes the greatest flow velocity from Bass Strait to Port Phillip Bay. This can be up to 1.5 m/s in the heads and up to approximately 0.5 m/s in a south easterly direction at the study site.
- Approximately mid-tide causes what is known as 'slack water' in the heads where very little to no tidal current is evident.

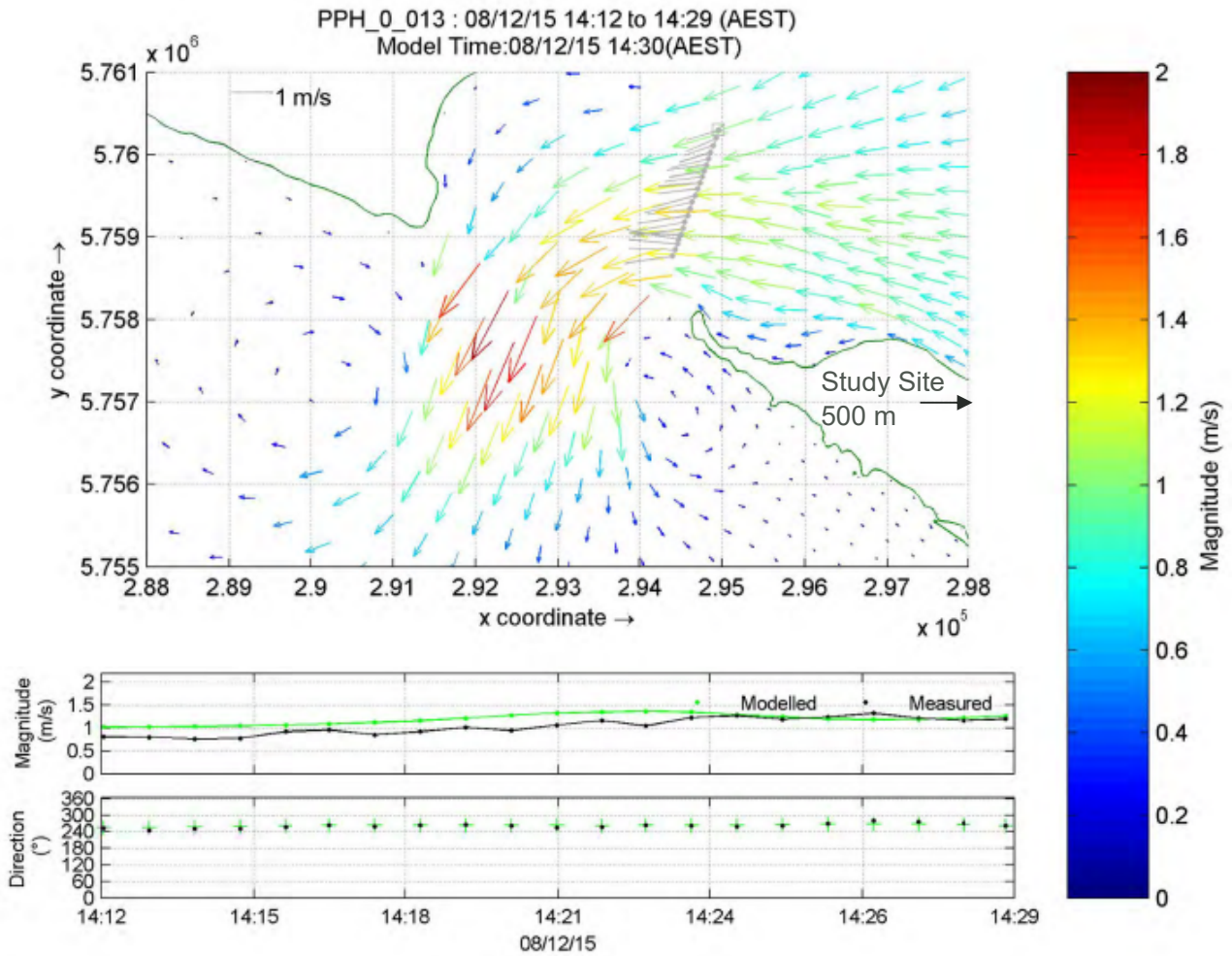


Figure 4.6 Ebb tide current velocities at Port Phillip heads (Cardno, 2017).

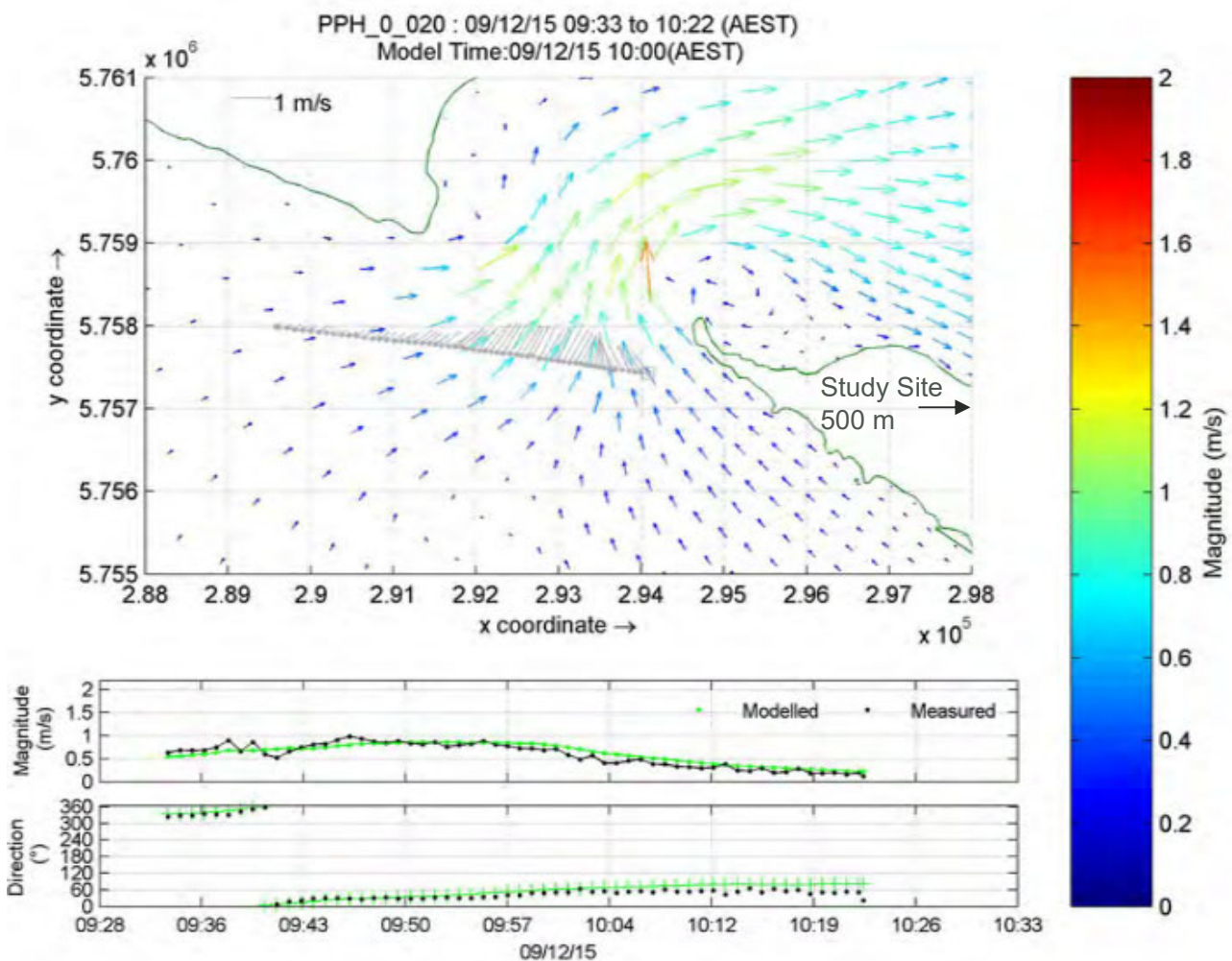


Figure 4.7 Ebb tide current velocities at Port Phillip heads (Cardno, 2017).

4.5 Sea Level Rise

The Intergovernmental Panel on Climate Change (IPCC) is the most widely recognised body that disseminates objective science on climate change and its associated impacts. The IPCC has released several broad documents that detail the state of the current science and prediction, the latest (finalised release) of which is the Special Report on the Ocean and Cryosphere in a Changing Climate (SROCC) (IPCC, 2019). The SROCC details the following conclusions:

- Mean sea level has risen globally throughout the 20th century and has accelerated in recent decades.
- The global mean sea level rise from 1902 to 2015 is 0.16m (likely range of 0.12-0.21m).
- The rate of sea level rise over 2006-2015 is 3.6mm/year (very likely range of 3.1-4.1mm/year).
- The Greenland and Antarctic ice sheets are predicted to lose mass at an increasing rate throughout the 21st century.
- Strong reductions in greenhouse gas emissions in the coming decades are required to reduce further changes after 2050.

These projected changes (last two points above) are based on a range of different global climate models that simulate several potential future scenarios of carbon emissions. These different scenarios

are known as Representative Concentration Pathways (RCPs). While it is currently difficult to predict the pathway that the global society will ‘adopt’ over the longer-term, these different RCPs provide suitable pathways to quantify potential impacts that would result for each one.

The most conservative ‘RCP8.5’ is typically adopted for coastal management planning in Australia. This represents a ‘business as usual’ pathway where limited success is achieved in reducing global carbon emissions. The projected rate of sea level rise for the Mornington Peninsula under the RCP8.5 scenario is illustrated in Figure 4.8.

Mornington Peninsula, Vic

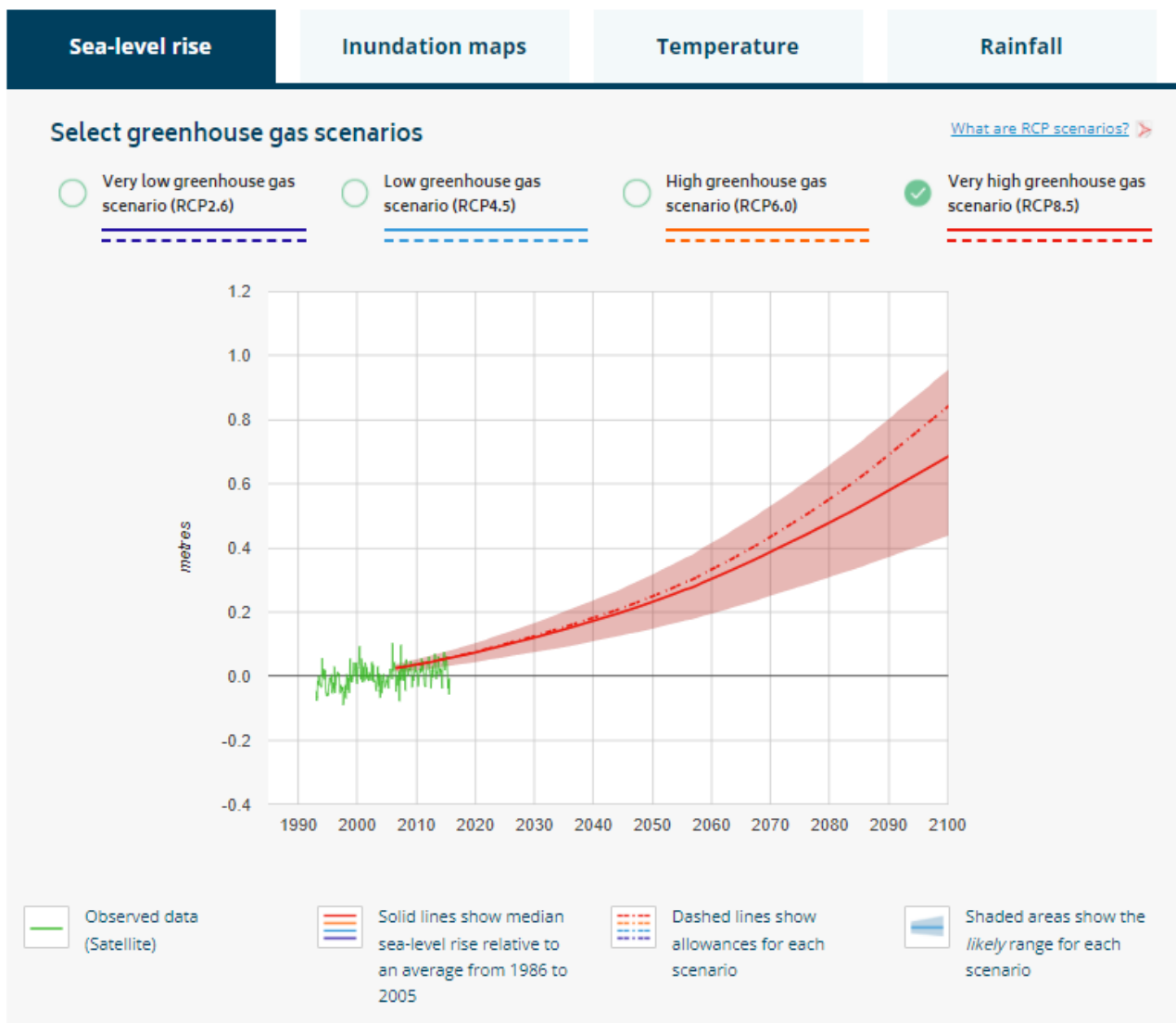


Figure 4.8 Sea Level Rise prediction, Mornington Peninsula (NCCARF, 2021)

The Victorian Marine and Coastal Policy (DELWP, 2020) requires coastal adaptation works to plan for "not less than 0.8 m" of sea level rise by 2100 relative to 2020 mean sea levels, which is within the likely range of the RCP8.5 scenario. Considering the sea level rise projection shown in Figure 4.8 and the Victorian Government policy, Table 4.3 presented the sea level rise allowances adopted for this study.

Table 4.3 Sea Level Rise under RCP8.5 emissions scenario scaled up to match the requirements of the VMACP (0.8 m by 2100).

Year	Sea Level Rise Allowances (m)
2030	0.2
2050	0.4
2070	0.5
2090	0.7
2100	0.8

4.6 Sediment Transport/Sand Movement

Sediment transport along the bay side coast of Point Nepean is driven by a combination of tidal currents through Port Phillip Heads, swell waves refracting around Point Nepean and breaking on beaches, and wind waves from the north breaking on beaches (Bird, Changes on the Coastline of Port Phillip Bay, 2011). These processes yield sediment transport with the following characteristics:

- West-east net sediment transport
- A fluctuating wide to narrow beach at the study area between approximately 12 – 0 m wide.



Figure 4.9 Panel A: Wide beach at old storm water outlet pipe 1973. Panel B: No high-tide beach shoreward of seawall 1957. Images: (Bird, 2011)

Recent studies into the erosion of Portsea front beach have focussed on sediment transport processes in the area. Modelling of the sediment driving processes described above resulted in an estimate of net sediment transport at Police Point (900m east of the current study area) of 2,000 m³/year in an easterly direction toward Portsea (Figure 4.10) (Advisian, 2016). This estimation is based on an aerial imagery analysis of changes in beach alignment within Weeroona Bay (Portsea) over one decade of available imagery from 2005-2015. It is intended to be indicative only. Further investigation is required to confirm the applicability of this estimation to the current study area.

We also note that gross sediment transport is likely of much greater magnitude than net sediment transport due to the relatively high velocity tidal currents along the Ticonderoga Bay coastline which may mobilise sand along the coast on ebb and flood tides.

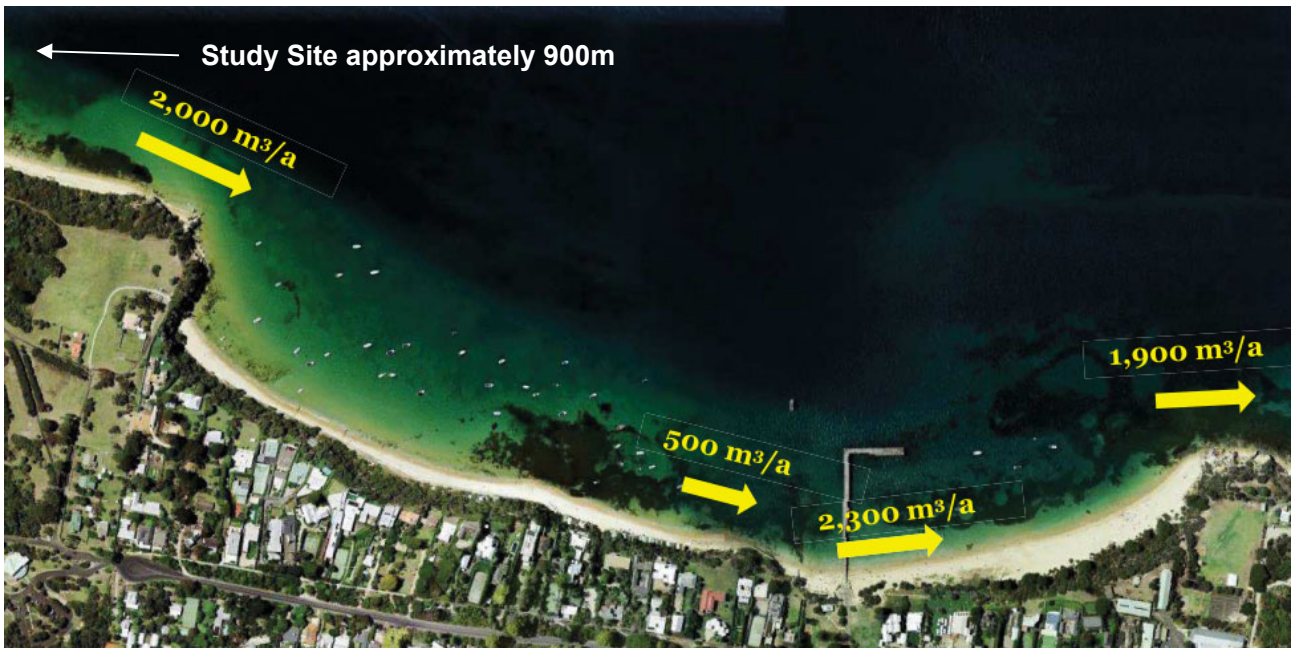


Figure 4.10 Net Sediment transport estimation based on aerial imagery analysis of images taken 2005-2015 in Weeroona bay Portsea (Advisian, 2016).

We analysed the bathymetry in the nearshore area adjacent to the proposed location for the PNREFS using a 2.5m bathymetry DEM (DELWP, 2022). A cross section of this coastal DEM through the approximate location of the HDD shown in Figure 4.11 shows a narrow area of shallow water immediately offshore from the beach (approximately 45 m wide). This shallow area has a steep drop-off on the seaward side with depth increasing to approximately 20m over a horizontal distance of 50m.

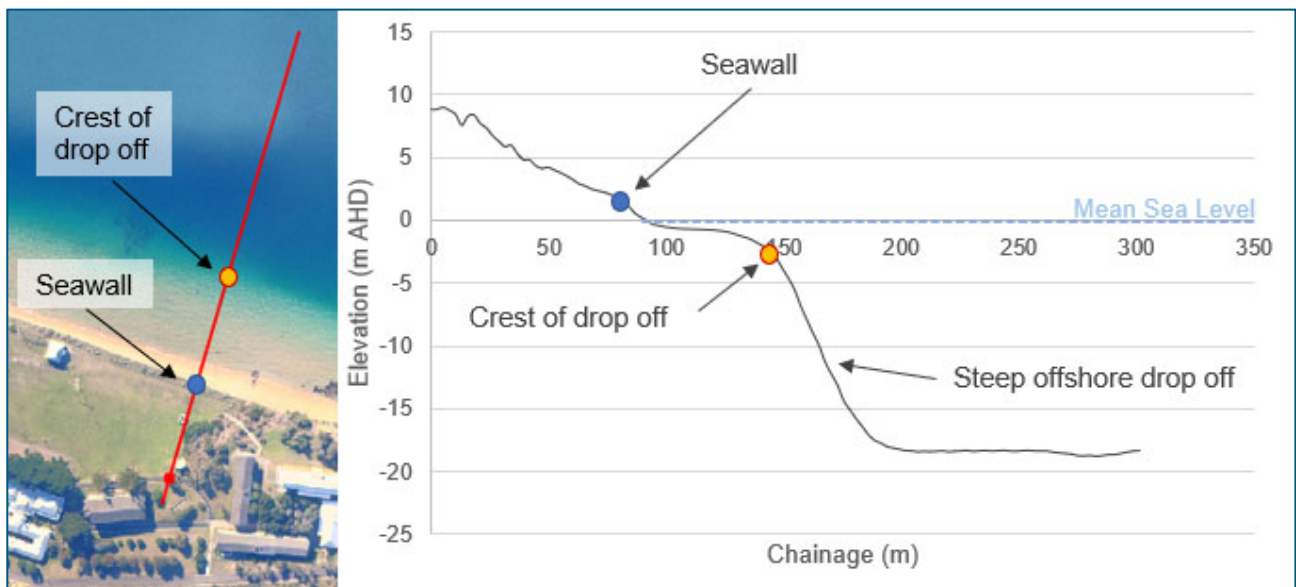


Figure 4.11 Topographic and bathymetric cross section of approximate HDD location on 2010 aerial image.

The bathymetry data also allowed us to identify that the change in colour seen in the aerial images between light green (shallow water) and darker blue (deeper water) represents the crest of the steep offshore drop off. Using this information, we assessed how the position of the offshore drop off relative

to the shoreline has changed over time by mapping the location of the drop off crest from 28 aerial images spanning 72 years (1950-2022). Results of this analysis demonstrate that the crest of the drop off has migrated landward and seaward approximately 15 m during the 72 years assessed. The current position of the crest is almost exactly in the middle of this zone of migration. There was no clear pattern between the year an image was taken and the position of the drop off crest relative to the shoreline.

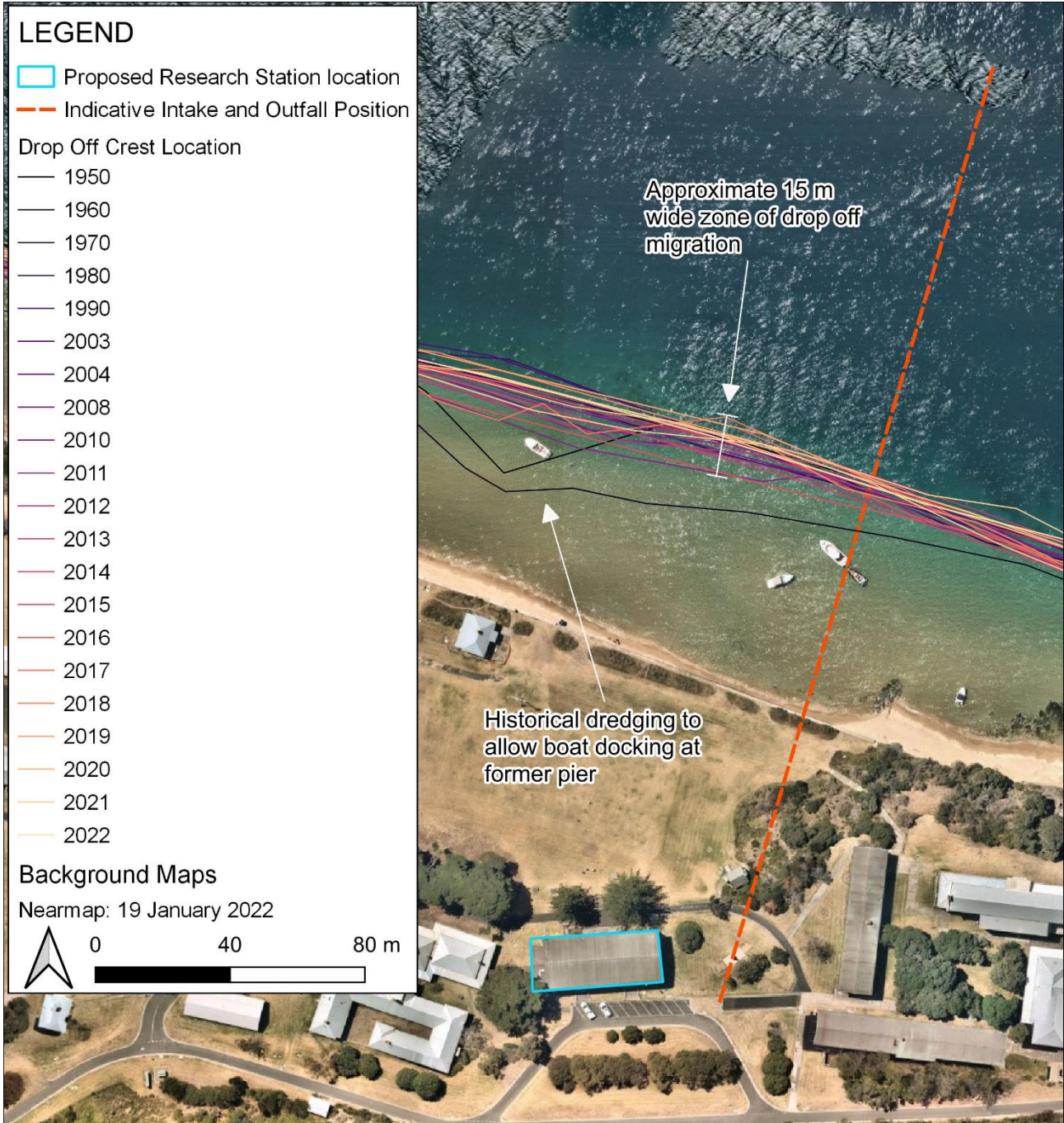


Figure 4.12 Offshore drop off zone of migration.

4.7 Predicted Impact of proposed works on coastal processes

Offshore works relating to the PNREFS which may have an impact on coastal processes are as follows:

- HDD of both the intake and outfall pipelines beneath the land and seabed surfaces
- Small scale excavation of the seabed using air lifting techniques to uncover the seaward end of the intake and outfall pipelines.
- Placement of pre-cast concrete structures on the seabed to encase the intake and outfall pipelines
- Placement of concrete scour protection mattresses around the intake and outfall to minimise scour around the pipes

None of these works or structures are considered likely to impact on coastal processes in the area. We note that the location of the outfall has been designed to avoid the drop off zone of migration and will sit on the relatively flat seabed at approximately 18-20m AHD.

5 Marine Ecology Impact Assessment

5.1 Description of Marine Environment

Overview and Protected Areas

The coastal area of the site is largely exposed, characterised by large windswept areas and steep coastal headlands. The site figures within larger ecologies and wildlife corridors—the entrance of Port Phillip Bay forms the eastern distribution limit for cold water species from Western Victoria and the western limit for warm water species from Eastern Victoria.

The sites' adjacent waters have been protected since 1975 and were proclaimed part of Port Phillip Heads Marine National Park (PPHMNP) in 2002 under the *National Parks Act 1975*. The two key parts of the PPHMNP include Point Nepean (waters adjacent to the headland) and the Portsea Hole (rectangular feature 600m offshore from the Portsea Pier) (Figure 5.1). Parks Victoria's *Port Phillip Heads Marine National Park Management Plan 2006* outlines conservation objectives, regulation of the park's recreational use, and the role of Parks Victoria in collaboratively managing the park. It is noted that the current project site is not within the extent of either marine national park. Each marine National Park is located approximately 1km from the project site.

The significant natural value of the PPHMNP at Point Nepean include (Parks Victoria, 2006):

- dynamic sedimentation regime related to the tidal and wave movement of sand through Port Phillip Heads, with sustained sandy accretion at Observatory Point
- an unusual example of a shore platform that has developed in contrasting wave environments, highlighting the processes that have shaped the opening to Port Phillip
- intertidal reef platforms that contain a high invertebrate diversity similar to Point Lonsdale which have been protected due to long-standing park regulations that prohibit access
- subtidal reefs with diverse fish and invertebrate assemblages with extensive encrusting communities such as ascidians, bryozoans and sponges
- frequent sightings of dolphin pods
- shorebird habitat along the reef platforms and sandy beaches.

The significant natural value of the PPHMNP at Portsea Hole include (Parks Victoria, 2006):

- unusual geomorphology consisting of a remnant section of the Yarra River, with a sharp gradient between the depths of 12 and 32m exposing strata changes over depth, descending from a limestone structure to a sandy base
- abundant and diverse fish assemblages and a rich benthic community of encrusting algae, sedentary organisms, sponges and soft corals.

The site is also adjacent the Ticonderoga Bay Sanctuary Zone (Figure 5.1), with frequent sighting of dolphins. This may include a unique species known as the Burrunan dolphin (*Tursiops australis*) as listed under the FFG Act. The Ticonderoga Bay Sanctuary Zone is protected under Section 16(1) of the *Wildlife (Marine Mammals) Regulations 2019* as a whale sanctuary zone (Victorian State Government, 2019). This regulation provides measures for exclusion zones and vessel restrictions in these sanctuary zones and near whales and dolphins.

The terrestrial area is classified as Point Nepean National Park, highlighting the site’s archaeological, ecological, architectural, historical, scientific and social significance.

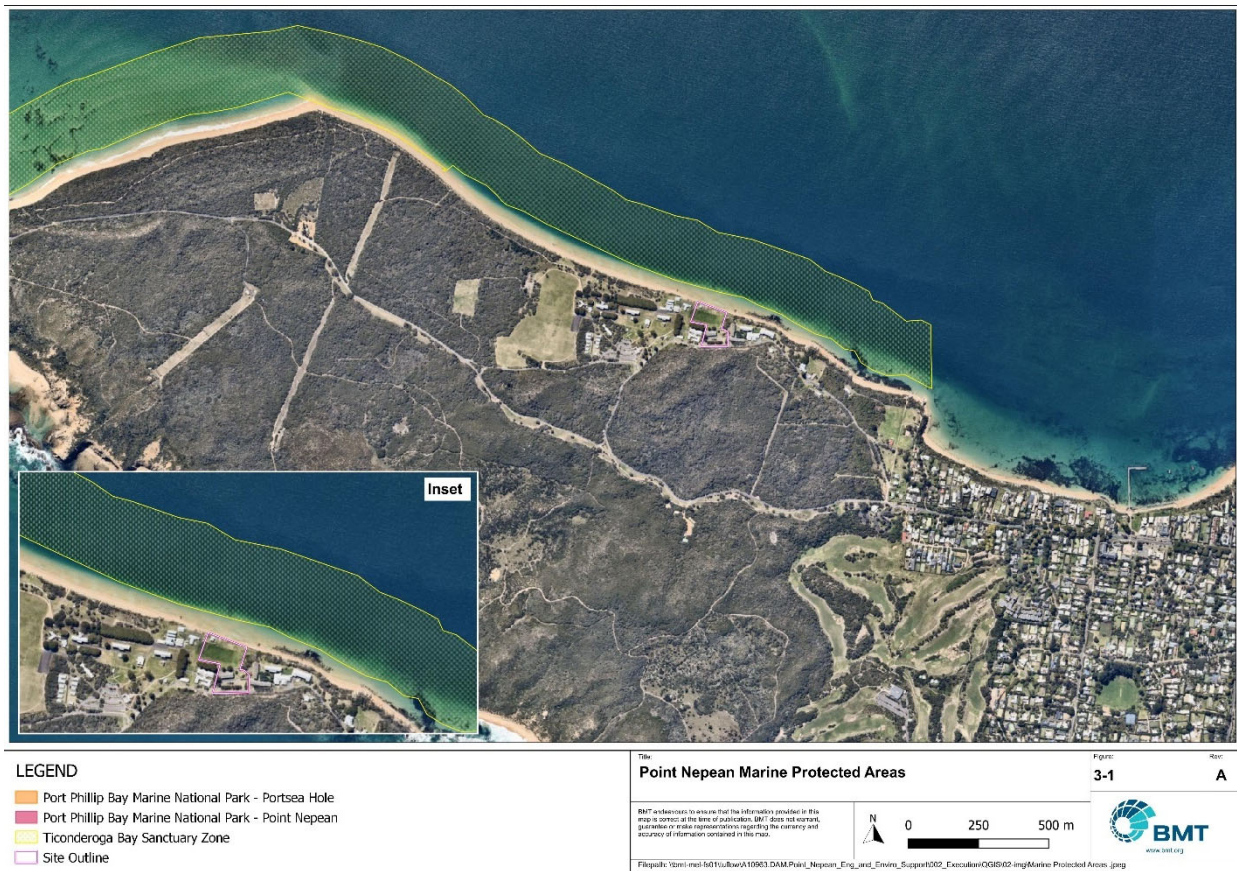


Figure 5.1 Point Nepean Marine Protected Areas

Benthic Habitats

A review of publicly available benthic habitat mapping indicates that there are potentially sensitive habitats directly to the east of the project site including seagrass and rocky reef and infralittoral rock (Figure 5.2). This mapping suggests the majority of benthic habitat is sand directly adjacent to the project site. The Portsea Hole is also classified as an area of coarse sediment. The data shown in Figure 5.2 is sourced from satellite imagery and field survey data.

This suggests that sensitive habitats including seagrass and hard substrate/corals may exist scattered around the project area. However, the information available indicates that it is unlikely that any of these sensitive habitats occurs neither at the specific site for the proposed intake and outfall nor in its direct proximity. The extent of seagrass will likely be limited to depths of less than 7m, which is where the majority of seagrass, predominantly *Zostera* spp. is located in the southern regions of Port Phillip Bay (Ball, Soto-Berelov, & Young, 2014).

Nevertheless, site-specific benthic habitat information is being gathered by Marine Ecology Australia for the purpose of this project, to better inform the final design and construction. In terms of impact assessment, it is expected that the site-specific information will not result in identification of higher environmental risk, but potentially lower, and importantly, the information will help informing and preparation of the environmental management plan for the construction stage.

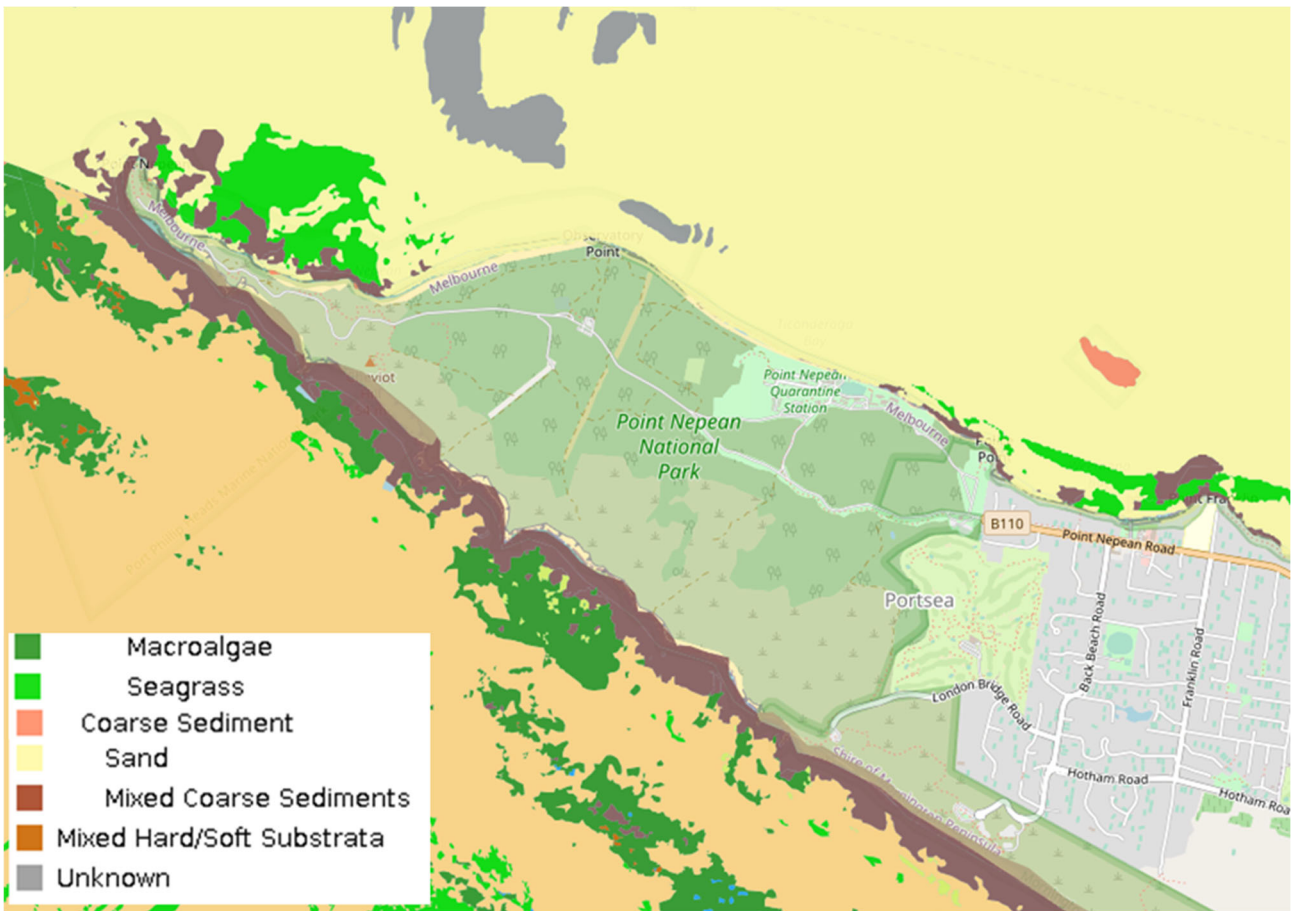


Figure 5.2 Benthic Habitats Source: (Seamap, 2021)

Threatened Species

Species lists from the FFG Act and EPBC Act threatened species list for the area were compared against observation and potential occurrence databases from the Atlas of Living Australia (Atlas of Living Australia, 2021) for the offshore area of the project site. Table 5.1 displays these species including conservation status for each legislative instrument. Atlas of Living Australia Records are shown in Annex A.

Table 5.1 Threatened Species

Common Name	Scientific Name	Atlas of Living Australia Record	FFG Act Status	EPBC Act Status	Likelihood of Presence in Study Area
Bird					
Fork- Tailed Swift	<i>Apus pacificus</i>	Potential	-	Marine/Migratory	Unlikely – mostly flies, unlikely to be present on sea surface or land
Marine Mammal					
Leopard Seal	<i>Hydrurga leptonyx</i>	Observed	-	Marine	Unlikely – predominately

Common Name	Scientific Name	Atlas of Living Australia Record	FFG Act Status	EPBC Act Status	Likelihood of Presence in Study Area
					Antarctic species. Seen in Victoria 3-5 times a year.
Burrnunan Dolphin	<i>Tursiops australis</i>	Observed	Critically Endangered	-	Possible – these dolphins are regularly seen in the T. Bay dolphin sanctuary.
Southern Right Whale	<i>Eubalaena australis</i>	Potential	Endangered	Endangered	Unlikely – predominantly pelagic species
Turtle					
Loggerhead Turtle	<i>Caretta caretta</i>	Potential	-	Endangered (Marine/Migratory)	Unlikely - predominantly tropical and subtropical species
Green Turtle	<i>Chelonia mydas</i>	Potential	-	Vulnerable (Marine/Migratory)	Unlikely - predominantly tropical and subtropical species
Leatherback Turtle, Leathery Turtle	<i>Dermochelys coriacea</i>	Potential	Critically Endangered	Endangered (Marine/Migratory)	Unlikely - predominantly tropical and subtropical species
Shark					
Great White Shark	<i>Carcharodon carcharias</i>	Potential	Endangered	Vulnerable (Migratory)	Possible – have been seen in Port Phillip Bay
Bony Fish					
Southern Bluefin Tuna	<i>Thunnus maccoyii</i>	Potential	Conservation Dependent	Conservation Dependent	Unlikely - predominantly pelagic species
Blue Warehou	<i>Seriolella brama</i>	Potential	Conservation Dependent	Conservation Dependent	Unlikely - predominantly pelagic species

Common Name	Scientific Name	Atlas of Living Australia Record	FFG Act Status	EPBC Act Status	Likelihood of Presence in Study Area
Southern Dogfish, Endeavour Dogfish, Little Gulper Shark	<i>Centrophorus zeehaani</i>	Potential	-	Conservation Dependent	Unlikely – predominantly deep-water species (i.e., >200m depth)

Marine Water Quality

The protection and management of surface and groundwaters is implemented under the *State Environmental Protection Policy (Waters) 2018* (SEPP Waters). Table 3.2 shows the quality indicators and objectives for Port Phillip Bay within the 'Exchange', as per the SEPP (Waters).

Table 5.2 Environmental Quality Indicators and Objectives for Port Phillip Bay – Exchange

Indicators	Percentile	Value
Surface/Bottom	-	Surface
Total phosphorus (µg/L)	75 th	50
Total nitrogen (µg/L)	75 th	150
Dissolved Oxygen (% saturation)	25 th - Max	N/A
Chlorophyll-a (µg/L)	75 th	1
Dissolved Inorganic Phosphorus (µg/L)	75 th	30
Dissolved Inorganic Nitrogen (µg/L)	75 th	10
Total Suspended Solids (mg/L)	75 th	2
Salinity (PSU)	25 th to 75 th	35-36
Light Attenuation (m-1)	75 th	0.3
pH	25 th to 75 th	7.5-8.5
Toxicants Water	-	99% Protection
Toxicants Sediment	-	Low

Available Marine Water Quality Information

Few water quality testing results are available for seawater at or near Point Nepean. The most relevant available data is from EPA testing of water quality at popular beaches through each summer period (EPA Victoria, 2021). This testing regime focused on the presence of microbial pathogens in sea water and found the following:

- During 2020-21 summer at Portsea front beach (closest testing location to current study site) there were 0 tested days with short-term objective levels over the SEPP (waters) standard of 200 orgs per 100mL.

There are no other water quality data available and further water quality measurements may need to be taken to determine baseline water quality characteristics for the current study.

5.2 Predicted Impact of Proposed Works on Marine Environment

Marine Ecology and Environment

Port Phillip Heads Marine National Park

The proposed locations of the intake and outfall are approximately 1km from both the main body of the PPHMNP and the Portsea Hole section of the park. Given effluent from the outfall is modelled to disperse fully within 25m of the outfall, the effluent is very unlikely to have any effect on the Marine National Park.

Similarly, any direct impacts of the outfall and intake structures on the marine environment will be localised to each location and will not have any impact on the Marine National Park.

Seagrass and Hard Substrate/Corals – Sensitive Benthic Habitats

Analysis of benthic habitat mapping showed small areas of seagrass and hard substrate (likely sub-tidal reef) present to the east of the study area, and that the seabed immediately offshore of the study area was predominantly sandy. This was confirmed via aerial image analysis which showed large areas of bare sand offshore of the offshore drop off.

The intake and outfall are situated to be on bare sandy flats with no sensitive benthic habitats within 25m of the outfall. As such, the construction and placement of the intake and outfall will have no direct impact on any sensitive areas, nor will the effluent from the outfall impact any sensitive habitats.

Threatened Species

All threatened species mapped as possibly being within the study area were identified as unlikely to be present except for the following:

- Burunan dolphin (*Tursiops australis*) – These dolphins are regularly observed in the Ticonderoga Bay Dolphin Sanctuary. They are highly mobile mammals and if present within the study area during the commencement of construction, would be expected to be naturally inclined to move away from the works area to avoid disturbance. Operation of the intake and outfall system is not expected to have any potential negative impacts to these animals.
- Great white shark (*Carcharodon carcharias*) – Great white sharks have been observed in Port Phillip Bay previously and may be in the vicinity of the study area at the commencement of construction. They are similarly highly mobile and would be expected to be naturally inclined to move away from the works area to avoid disturbance.

As such, the proposed works are not likely to pose any risk to threatened species possibly present within the study area.

Water Quality

HDD Drilling of intake and outfall

There are a variety of potential risks predominantly to water quality from HDD drilling as follows:

- **Frac Outs of Drilling Fluid** – When HDD drilling occurs, a lubricating fluid is used which is usually either a saltwater polymer fluid, or a natural bentonite clay slurry. Neither of these fluids are toxic and bentonite clay is a naturally and commonly occurring substance. HDD drilling is performed under pressure which can cause some of the drilling fluid to escape the borehole (either through loose soil in a shallow bore or through a fracture or fault in the geology) and reach the surface – termed a Frac Out. These fluids are non-toxic, however, if frac outs occur beneath the sea surface and a clay slurry is used, a sediment plume may occur which may cause concerns amongst the

community. The marine environment within the study area is naturally very dispersive which means that any fluid escaping the bore would be dispersed below background levels very quickly and would not likely settle to the sea floor. As such, there is a low risk that frac outs will cause negative impacts to water quality or the marine environment if they occur. Measures to avoid Frac Outs where possible during the construction process will be utilised as outlined in Section 6 and Annex B below.

- **Loss of drilling fluids and bore cuttings from the offshore end of the bore** – Some HDD methods can cause loss of the drilling fluid and borehole cuttings from the offshore end of the bore. As above, this may cause a sediment plume beneath the sea surface from the escaping drilling fluid (if a clay slurry is used) and may cause concerns within the community. Any plume will likely disperse very quickly in the high energy marine environment and will not likely have any negative impacts on water quality or the marine environment. Bore hole cuttings are waste material (coarse rock cuttings/shavings from the drilling process). At the end of the borehole drilling process, the hole is cleaned of these cuttings. If the hole is drilled completely through to the ocean, these cuttings will be flushed out the ocean end of the pipe and settle on the seafloor. The benthic environment surrounding the proposed bore hole end is a sandy plain and rock cuttings falling to the sea floor here will not likely pose a risk to benthic flora or fauna. Different HDD methods can be used to prevent the escape of both drilling fluid and bore cuttings from the end of the pipe to further reduce/mitigate any potential impacts as described in Section 6 and Annex B below.

Outfall Effluent

The effluent emitted from the outfall will be seawater with elevated nutrient concentrations from excess feedstock and dissolved fauna excrement. Near field modelling of effluent dispersal demonstrates that even during slack tides (the time period with the lowest hydrodynamic energy at this site), effluent will likely disperse fully within 25m of the outfall. As such, the effluent is predicted to have minimal impact on water quality in the area.

6 Potential Impact Summary and Recommendations

The possible impacts of the proposed outfall and intake pipe works are summarised in Table 6.1 below. Key risks for consideration by University of Melbourne and Monash University are the potential for frac-outs of HDD drilling fluid within the Ticonderoga Bay Dolphin Sanctuary, and the loss of drilling fluids and bore cuttings from the end of the pipe during pipe reaming and flushing. Recommendations to control and mitigate these risks are outlined in Table 6.1 below.

Table 6.1 Risk Summary

Category / Activity	Potential Impact	Risk Rating	Mitigation Recommendations
Horizontal Directional Drilling (HDD) of intake and outfall	Frac-out of drilling fluid potentially producing sediment plume (if using natural clay slurry fluid)	Low Risk	<p>Report any frac-out occurrence to relevant regulator within 2 hours of occurrence.</p> <p>Perform any required geotechnical investigations prior to HDD as guided by technical specialist contractor.</p> <p>Use the most appropriate drilling fluid as recommended by technical specialists.</p> <p>Monitor drilling fluid properties during drilling process.</p> <p>Monitor pressure of the HDD bore hole during drilling.</p>
Horizontal Directional Drilling (HDD) of intake and outfall	Discharge of drilling fluid and bore cuttings from the offshore end of the bore	Low Risk	<p>Leave the bore closed just prior to exit from seabed while hole is successively reamed and flushed of cuttings (back to the landside).</p> <p>Thrust the pipe into the bore from the landside to the still closed offshore end of the bore.</p> <p>Shallow seabed excavation down to the inserted pipe using air lifting of seabed sediment material.</p>
Outfall effluent discharge - Water Quality	Potential impact of seawater system outfall effluent discharge on water quality in the receiving marine environment of Port Phillip Bay	Low Risk	Regular monitoring and reporting to EPA) of effluent stream characteristics according to conditions to be set out in future EPA Operating License or License Exemption.

All potential impacts on coastal processes and to the marine environment from the proposed Point Nepean Research and Education Field Station seawater intake and outfall system have low (or very low) risk of potential impact. Measures for further risk mitigation regarding the HDD process for the intake and outfall pipes are described in Table 6.1 and these should be used as guided by HDD specialist contactors.

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Annex A Atlas of Living Australia Survey Records

authority	family	geom_idx	desmaltl	group_name	family_lid	pid	endemic	genus_name	estuarine	fl	coastal	fl	scientific	specific_n	imageUrl	genus_lid	lid	metadata_u_type	area_name	data_resourc	depth	intersectArea	image_qualit	common_name	caab_family_spcode	max_depth	gd	caab_species_pelagic	wmsurl
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34022	Klunzinger, BOOTHIDAE	34485	TRUE	Arngnolus	704519	FALSE	FALSE	Arngnolus muelleri					muelleri			704519	704519	FALSE	Arngnolus muelleri	Expert distrib d803	5	28 377	3746003	200	1004	37 46003	0	https://spatial.ala.org.au/geosevier/ams?service=WMS&version=1.0.0&request=GetMap&layers=ALA.Distributions&format=png&viewparams=34022	
34890	Gyther, 18 BOOTHIDAE	34485	TRUE	Arngnolus	704837	FALSE	FALSE	Arngnolus muelleri					muelleri			704837	704837	FALSE	Arngnolus muelleri	Expert distrib d803	11	28 378	3746001	60	1869	37 46001	0	https://spatial.ala.org.au/geosevier/ams?service=WMS&version=1.0.0&request=GetMap&layers=ALA.Distributions&format=png&viewparams=34890	
33165	McCulloch, EBRAHONCHTHYIDAE	34485	TRUE	Thymichthys	704662	FALSE	FALSE	Thymichthys verrucosus					verrucosus			704662	704662	FALSE	Thymichthys verrucosus	Expert distrib d803	5	28 379	3729001	230	143	37 29001	0	https://spatial.ala.org.au/geosevier/ams?service=WMS&version=1.0.0&request=GetMap&layers=ALA.Distributions&format=png&viewparams=33165	
33312	Schwartzbars, THYTHIDAE	34485	FALSE	Dactylosaurus	704609	TRUE	FALSE	Dactylosaurus gomoni					gomoni			704609	704609	FALSE	Dactylosaurus gomoni	Expert distrib d803	0	28 378	3722804	25	289	37 22804	0	https://spatial.ala.org.au/geosevier/ams?service=WMS&version=1.0.0&request=GetMap&layers=ALA.Distributions&format=png&viewparams=33312	
36136	Gyther, 18 CALLANTHIDAE	34485	TRUE	Callanthus	704953	TRUE	FALSE	Callanthus aliporhi					aliporhi			704953	704953	FALSE	Callanthus aliporhi	Expert distrib d803	15	31 311	3731004	365	317	37 31004	1	https://spatial.ala.org.au/geosevier/ams?service=WMS&version=1.0.0&request=GetMap&layers=ALA.Distributions&format=png&viewparams=36136	
3725	Gyther, 18 CALLANTHIDAE	34485	TRUE	Callanthus	704954	FALSE	FALSE	Callanthus aliporhi					aliporhi			704954	704954	FALSE	Callanthus aliporhi	Expert distrib d803	15	31 311	3731005	365	317	37 31005	1	https://spatial.ala.org.au/geosevier/ams?service=WMS&version=1.0.0&request=GetMap&layers=ALA.Distributions&format=png&viewparams=3725	
3094	Klunzinger, CAMPHEPHAGIDAE	34485	FALSE	Coracina	704241	FALSE	FALSE	Coracina (Edollonia) tenuirostris					tenuirostris			704241	704241	FALSE	Coracina (Edollonia) tenuirostris	Expert distrib d804				Cadabird	24870		24870	https://spatial.ala.org.au/geosevier/ams?service=WMS&version=1.0.0&request=GetMap&layers=ALA.Distributions&format=png&viewparams=3094	
30620	River, 1833 CARANGIDAE	34485	TRUE	Caranx	705011	FALSE	FALSE	Caranx georgianus					georgianus			705011	705011	FALSE	Caranx georgianus	Expert distrib d803	11	28 377	3737004	124	167	37 37004	0	https://spatial.ala.org.au/geosevier/ams?service=WMS&version=1.0.0&request=GetMap&layers=ALA.Distributions&format=png&viewparams=30620	
36716	River, 1833 CARANGIDAE	34485	TRUE	Prædicrostus	705013	FALSE	FALSE	Prædicrostus georgianus					georgianus			705013	705013	FALSE	Prædicrostus georgianus	Expert distrib d803	0	28 377	3737002	200	3698	37 37002	1	https://spatial.ala.org.au/geosevier/ams?service=WMS&version=1.0.0&request=GetMap&layers=ALA.Distributions&format=png&viewparams=36716	
36612	Richardson, 13 CARANGIDAE	34485	TRUE	Trachurus	705015	FALSE	FALSE	Trachurus medius					medius			705015	705015	FALSE	Trachurus medius	Expert distrib d803	0	28 377	3737003	200	3698	37 37003	1	https://spatial.ala.org.au/geosevier/ams?service=WMS&version=1.0.0&request=GetMap&layers=ALA.Distributions&format=png&viewparams=36612	
36619	Richardson, 13 CARANGIDAE	34485	TRUE	Trachurus	705016	FALSE	FALSE	Trachurus novaezelandiae					novaezelandiae			705016	705016	FALSE	Trachurus novaezelandiae	Expert distrib d803	0	28 377	3737003	200	3616	37 37003	1	https://spatial.ala.org.au/geosevier/ams?service=WMS&version=1.0.0&request=GetMap&layers=ALA.Distributions&format=png&viewparams=36619	
32764	Centrolophidae	34485	FALSE	Seriola lalandi	704629	FALSE	FALSE	Seriola lalandi					lalandi			704629	704629	FALSE	Seriola lalandi	Expert distrib d2099				Blue Warehouse	26974		33768	https://spatial.ala.org.au/geosevier/ams?service=WMS&version=1.0.0&request=GetMap&layers=ALA.Distributions&format=png&viewparams=32764	
4394	CHARADRIIDAE	34485	FALSE	Charadrius	704191	FALSE	FALSE	Charadrius (Charadrius) leschenaulti					leschenaulti			704191	704191	FALSE	Charadrius (Charadrius) leschenaulti	Expert distrib d804				Greater Sand Plover	24720		24720	https://spatial.ala.org.au/geosevier/ams?service=WMS&version=1.0.0&request=GetMap&layers=ALA.Distributions&format=png&viewparams=4394	
4395	CHARADRIIDAE	34485	FALSE	Charadrius	704192	FALSE	FALSE	Charadrius (Charadrius) monguilloti					monguilloti			704192	704192	FALSE	Charadrius (Charadrius) monguilloti	Expert distrib d804				Lesser Sand Plover	24721		24721	https://spatial.ala.org.au/geosevier/ams?service=WMS&version=1.0.0&request=GetMap	

Richardson, MURAENIDAE	34973	FALSE	eels	urn:isid:biodi	7048470	FALSE	Gymnothorax	FALSE	TRUE	Gymnothorax prasinus	https://spati.urn.isid.biodi.urn.isid.biodi/http://www.e	Expert distrib d#903	0	2.8817E-05	E	Green Moray	37 060	3700006	40	1957 37 060006	0	https://spati.ala.org.au/govseer/wms?service=WMS&version=1.1.0&request=GetMap&layers=ALA.Distributions&format=image/png&viewparams=34973
Muridae	31331	TRUE	rays	urn:isid:biodi	7044828	FALSE	Pseudomy	FALSE	TRUE	Pseudomy novaeholandiae	https://spati.urn.isid.biodi.urn.isid.biodi/http://www.e	Expert distrib d#2099	0	2.8817E-05	E	New Holland Mouse, Poekila	37 009	20096	130	3458	0	https://spati.ala.org.au/govseer/wms?service=WMS&version=1.1.0&request=GetMap&layers=ALA.Distributions&format=image/png&viewparams=31331
Hector, 1877 MYLIOBATIDAE	34757	TRUE	rays	urn:isid:biodi	7048254	FALSE	Myliobatis	FALSE	TRUE	Myliobatis tenuicaudatus	https://spati.urn.isid.biodi.urn.isid.biodi/http://www.e	Expert distrib d#803	0	2.8817E-05	E	Southern Eagle Ray	37 039	3703901	130	1737 03901	0	https://spati.ala.org.au/govseer/wms?service=WMS&version=1.1.0&request=GetMap&layers=ALA.Distributions&format=image/png&viewparams=34757
Neobolabinae	30428	TRUE		urn:isid:biodi	7043925	FALSE	Capena	FALSE	TRUE	Capena marginata	https://spati.urn.isid.biodi.urn.isid.biodi/http://www.e	Expert distrib d#2099	0	2.8817E-05	E	Pigmy Right Whale	37 039	10039	130	32802	0	https://spati.ala.org.au/govseer/wms?service=WMS&version=1.1.0&request=GetMap&layers=ALA.Distributions&format=image/png&viewparams=30428
Whitley, 1893 OPHIURIDAE	33255	TRUE		urn:isid:biodi	7049752	FALSE	Gerypteris	FALSE	TRUE	Gerypteris bicoloris	https://spati.urn.isid.biodi.urn.isid.biodi/http://www.e	Expert distrib d#803	10	2.8817E-05	E	Pink Gurnard Perch	37 228	3722802	46	2529 22802	0	https://spati.ala.org.au/govseer/wms?service=WMS&version=1.1.0&request=GetMap&layers=ALA.Distributions&format=image/png&viewparams=33255
Guichenot, LINEOSEBASTIDAE	35648	TRUE		urn:isid:biodi	7049145	TRUE	Neosebastes	FALSE	TRUE	Neosebastes scorpaenoides	https://spati.urn.isid.biodi.urn.isid.biodi/http://www.e	Expert distrib d#803	1	2.8817E-05	G	Common Gurnard Perch	37 287	3728705	181	2628 28705	0	https://spati.ala.org.au/govseer/wms?service=WMS&version=1.1.0&request=GetMap&layers=ALA.Distributions&format=image/png&viewparams=35648
OCEANITIDAE	5378	TRUE		urn:isid:biodi	7042424	FALSE	Fregetta	FALSE	TRUE	Fregetta tropica	https://spati.urn.isid.biodi.urn.isid.biodiversity.org.au	Expert distrib d#804	2	2.8817E-05	G	Black-bellied Storm-petrel	37 287	25154	154	25154	0	https://spati.ala.org.au/govseer/wms?service=WMS&version=1.1.0&request=GetMap&layers=ALA.Distributions&format=image/png&viewparams=5378
OCEANITIDAE	6018	TRUE		urn:isid:biodi	7043064	FALSE	Oceanites	FALSE	TRUE	Oceanites oceanicus	https://spati.urn.isid.biodi.urn.isid.biodiversity.org.au	Expert distrib d#804	2	2.8817E-05	G	Wilson's Storm-petrel	37 287	25154	154	25154	0	https://spati.ala.org.au/govseer/wms?service=WMS&version=1.1.0&request=GetMap&layers=ALA.Distributions&format=image/png&viewparams=6018
OCEANITIDAE	6018	TRUE		urn:isid:biodi	7043174	FALSE	Dagapomoma	FALSE	TRUE	Dagapomoma australis	https://spati.urn.isid.biodi.urn.isid.biodiversity.org.au	Expert distrib d#804	2	2.8817E-05	G	Shortfin Storm-petrel	37 287	25154	154	25154	0	https://spati.ala.org.au/govseer/wms?service=WMS&version=1.1.0&request=GetMap&layers=ALA.Distributions&format=image/png&viewparams=6018
(Macleay, 1859) OPHIURIDAE	34897	TRUE	eels	urn:isid:biodi	7048394	FALSE	Scotelenchelys	FALSE	TRUE	Scotelenchelys australis	https://spati.urn.isid.biodi.urn.isid.biodi/http://www.e	Expert distrib d#803	2	2.8817E-05	G	Shortfin Worm Fel	37 068	3706803	50	1876 06803	0	https://spati.ala.org.au/govseer/wms?service=WMS&version=1.1.0&request=GetMap&layers=ALA.Distributions&format=image/png&viewparams=34897
(Günther, 1810) OPHIURIDAE	34898	TRUE	eels	urn:isid:biodi	7048395	FALSE	Scotelenchelys	FALSE	TRUE	Scotelenchelys brevicauda	https://spati.urn.isid.biodi.urn.isid.biodi/http://www.e	Expert distrib d#803	0	2.8817E-05	G	Shortfin Worm Fel	37 068	3706804	156	1877 06804	0	https://spati.ala.org.au/govseer/wms?service=WMS&version=1.1.0&request=GetMap&layers=ALA.Distributions&format=image/png&viewparams=34898
(McCulloch, 1910) OPHIURIDAE	34089	FALSE	eels	urn:isid:biodi	7048396	TRUE	Scotelenchelys	FALSE	TRUE	Scotelenchelys tasmaniensis	https://spati.urn.isid.biodi.urn.isid.biodi/http://www.e	Expert distrib d#803	0	2.8817E-05	A	Tasmanian Worm Fel	37 068	3708808	50	1878 06808	0	https://spati.ala.org.au/govseer/wms?service=WMS&version=1.1.0&request=GetMap&layers=ALA.Distributions&format=image/png&viewparams=34089
Linnaeus, 1793 OPHIURIDAE	33255	TRUE		urn:isid:biodi	7049752	FALSE	Gerypteris	FALSE	TRUE	Gerypteris bicoloris	https://spati.urn.isid.biodi.urn.isid.biodi/http://www.e	Expert distrib d#803	10	2.8817E-05	G	Pink Gurnard Perch	37 228	3722802	46	2529 22802	0	https://spati.ala.org.au/govseer/wms?service=WMS&version=1.1.0&request=GetMap&layers=ALA.Distributions&format=image/png&viewparams=33255
Klunzinger, 1 OPHIURIDAE	33166	TRUE		urn:isid:biodi	7046663	TRUE	Gerypteris	FALSE	TRUE	Gerypteris tigrinus	https://spati.urn.isid.biodi.urn.isid.biodi/http://www.e	Expert distrib d#803	1	2.8817E-05	E	Rock Ling	37 228	3722808	60	235 22808	0	https://spati.ala.org.au/govseer/wms?service=WMS&version=1.1.0&request=GetMap&layers=ALA.Distributions&format=image/png&viewparams=33166
Orchidaceae	31728	TRUE		https://id.bi	7045225	FALSE	Prasophyllum	FALSE	TRUE	Prasophyllum frenchii	https://spati.urn.isid.biodi.urn.isid.biodi/http://www.e	Expert distrib d#2099	1	2.8817E-05	E	Maroon Leek-orchid, Slaty Leek-orchid, Stout Leek-orchid, French's Leek-orchid, Swamp Leek-orchid	37 228	20974	34609	34609	0	https://spati.ala.org.au/govseer/wms?service=WMS&version=1.1.0&request=GetMap&layers=ALA.Distributions&format=image/png&viewparams=31728
Orchidaceae	31892	TRUE		https://id.bi	7045389	FALSE	Pterostylis	FALSE	TRUE	Pterostylis cucullata	https://spati.urn.isid.biodi.urn.isid.biodi/http://www.e	Expert distrib d#2099	5	4.4441E-06	G	Leafy Cowslip	37 466	21549	34629	34629	0	https://spati.ala.org.au/govseer/wms?service=WMS&version=1.1.0&request=GetMap&layers=ALA.Distributions&format=image/png&viewparams=31892
Shaw, 1798 OSTRACODAE	34698	TRUE		urn:isid:biodi	7047195	TRUE	Arcana	FALSE	TRUE	Arcana aurita	https://spati.urn.isid.biodi.urn.isid.biodi/http://www.e	Expert distrib d#803	10	2.8817E-05	G	White-faced Storm-petrel	37 228	3746603	160	680 3746603	0	https://spati.ala.org.au/govseer/wms?service=WMS&version=1.1.0&request=GetMap&layers=ALA.Distributions&format=image/png&viewparams=34698
McCulloch, 1910 OSTRACODAE	34188	TRUE	sharks	urn:isid:biodi	7047595	FALSE	Paracymulum	FALSE	TRUE	Paracymulum fernuginum	https://spati.urn.isid.biodi.urn.isid.biodi/http://www.e	Expert distrib d#803	5	2.8817E-05	G	Rusty Carapack Shark	37 013	3701304	150	1167 013005	0	https://spati.ala.org.au/govseer/wms?service=WMS&version=1.1.0&request=GetMap&layers=ALA.Distributions&format=image/png&viewparams=34188
(Günther, 1810) PATAEODAE	36121	TRUE		urn:isid:biodi	7049618	TRUE	Aetopus	FALSE	TRUE	Aetopus maculatus	https://spati.urn.isid.biodi.urn.isid.biodi/http://www.e	Expert distrib d#803	1	2.8817E-05	G	Warty Prowfish	37 292	3729204	45	3107 29204	0	https://spati.ala.org.au/govseer/wms?service=WMS&version=1.1.0&request=GetMap&layers=ALA.Distributions&format=image/png&viewparams=36121
(Castelnau, 1) PATAEODAE	36122	FALSE		urn:isid:biodi	7049619	TRUE	Neopateacus	FALSE	TRUE	Neopateacus waterhousei	https://spati.urn.isid.biodi.urn.isid.biodi/http://www.e	Expert distrib d#803	0	2.8817E-05	G	Whiskered Prowfish	37 292	3729205	40	3103 29205	0	https://spati.ala.org.au/govseer/wms?service=WMS&version=1.1.0&request=GetMap&layers=ALA.Distributions&format=image/png&viewparams=36122
Linnaeus, 1793 OSTRACODAE	35293	TRUE		urn:isid:biodi	7049773	TRUE	Pegasus	FALSE	TRUE	Pegasus latirostris	https://spati.urn.isid.biodi.urn.isid.biodi/http://www.e	Expert distrib d#803	10	2.8817E-05	G	Scalpedured Seahorse	37 309	3730903	55	3250 30903	0	https://spati.ala.org.au/govseer/wms?service=WMS&version=1.1.0&request=GetMap&layers=ALA.Distributions&format=image/png&viewparams=35293
McCulloch, 1910 OSTRACODAE	36885	TRUE		urn:isid:biodi	7050382	TRUE	Paracymulum	FALSE	FALSE	Paracymulum elongatus	https://spati.urn.isid.biodi.urn.isid.biodi/http://www.e	Expert distrib d#803	1	2.8817E-05	G	Elongate Bullseye	37 357	3735702	70	3869 35702	0	https://spati.ala.org.au/govseer/wms?service=WMS&version=1.1.0&request=GetMap&layers=ALA.Distributions&format=image/png&viewparams=36885
Klunzinger, 1 PEMPHRIDAE	36875	TRUE		urn:isid:biodi	7050372	TRUE	Pempheris	FALSE	TRUE	Pempheris multiradiata	https://spati.urn.isid.biodi.urn.isid.biodi/http://www.e	Expert distrib d#803	2	2.8817E-05	G	Bigscale Bullseye	37 357	3735701	70	3859 35701	0	https://spati.ala.org.au/govseer/wms?service=WMS&version=1.1.0&request=GetMap&layers=ALA.Distributions&format=image/png&viewparams=36875
Peramulidae	32740	TRUE		urn:isid:biodi	7046237	FALSE	Isodon	FALSE	TRUE	Isodon obesus obesus	https://spati.urn.isid.biodi.urn.isid.biodi/http://www.e	Expert distrib d#2099	1	2.8817E-05	G	Southern Brown Bandicoot (Eastern)	37 292	268050	34823	34823	0	https://spati.ala.org.au/govseer/wms?service=WMS&version=1.1.0&request=GetMap&layers=ALA.Distributions&format=image/png&viewparams=32740
Whitley, 1939 PERCOPHIDAE	37377	TRUE		urn:isid:biodi	7050874	FALSE	Enigmarperca	FALSE	TRUE	Enigmarperca reducta	https://spati.urn.isid.biodi.urn.isid.biodi/http://www.e	Expert distrib d#803	1	2.8817E-05	A	Brood Bucker	37 393	3739308	60	4354 3739308	0	https://spati.ala.org.au/govseer/wms?service=WMS&version=1.1.0&request=GetMap&layers=ALA.Distributions&format=image/png&viewparams=37377
OCEANITIDAE	5172	TRUE	flatheads	urn:isid:biodi	7042129	FALSE	Dagapomoma	FALSE	TRUE	Dagapomoma australis	https://spati.urn.isid.biodi.urn.isid.biodi/http://www.e	Expert distrib d#804	10	2.8817E-05	E	Touty Flathead	37 226	3722603	160	3220 22603	0	https://spati.ala.org.au/govseer/wms?service=WMS&version=1.1.0&request=GetMap&layers=ALA.Distributions&format=image/png&viewparams=5172
(Richardson, 1847) PLATYCEPHALIDAE	36098	FALSE	flatheads	urn:isid:biodi	7049595	TRUE	Thysanophrys	TRUE	TRUE	Thysanophrys cirronasa	https://spati.urn.isid.biodi.urn.isid.biodi/http://www.e	Expert distrib d#803	0	2.8817E-05	G	Tasleouth Flathead	37 296	3729605	35	3077 29605	0	https://spati.ala.org.au/govseer/wms?service=WMS&version=1.1.0&request=GetMap&layers=ALA.Distributions&format=image/png&viewparams=36098
McCoy, 1890 PLESIODAE	36332	FALSE		urn:isid:biodi	7049829	TRUE	Thrinacos	TRUE	TRUE	Thrinacos caudimaculatus	https://spati.urn.isid.biodi.urn.isid.biodi/http://www.e	Expert distrib d#803	2	2.8817E-05	G	Southern Hulafish	37 316	3731601	35	3305 31601	0	https://spati.ala.org.au/govseer/wms?service=WMS&version=1.1.0&request=GetMap&layers=ALA.Distributions&format=image/png&viewparams=36332
(Richardson, 1847) PLESIODAE	36099	TRUE	flatfishes	urn:isid:biodi	7047506	TRUE	Ammotretis	FALSE	TRUE	Ammotretis litaratus	https://spati.urn.isid.biodi.urn.isid.biodi/http://www.e	Expert distrib d#803	1	2.8817E-05	G	Spotted Flounder	37 461	3746104	100	987 3746104	0	https://spati.ala.org.au/govseer/wms?service=WMS&version=1.1.0&request=GetMap&layers=ALA.Distributions&format=image/png&viewparams=36099
(Linnaeus, 17) POMACENTRIDAE	34813	TRUE		urn:isid:biodi	7050110	FALSE	Pomatomus	TRUE	TRUE	Pomatomus saltatrix	https://spati.urn.isid.biodi.urn.isid.biodi/http://www.e	Expert distrib d#803	0	2.8817E-05	G	Tailor	37 364	3736402	15	3793 36402	1	https://spati.ala.org.au/govseer/wms?service=WMS&version=1.1.0&request=GetMap&layers=ALA.Distributions&format=image/png&viewparams=34813
(Günther, 18) PRISTIGORIDAE	34678	TRUE	sharks	urn:isid:biodi	7048175	TRUE	Pristiophorus	FALSE	TRUE	Pristiophorus nudipinnis	https://spati.urn.isid.biodi.urn.isid.biodi/http://www.e	Expert distrib d#803	5	2.8817E-05	G	Southern Sawshark	37 023	3702301	110	1655 02301	0	https://spati.ala.org.au/govseer/wms?service=WMS&version=1.1.0&request=GetMap&layers=ALA.Distributions&format=image/png&viewparams=34678
PROCELLARIDAE	6448	TRUE		urn:isid:biodi	7043094	FALSE	Ardenna	FALSE	TRUE	Ardenna carneipes	https://spati.urn.isid.biodi.urn.isid.biodiversity.org.au	Expert distrib d#804	2	2.8817E-05	G	Flash-footed Shearwater	37 228	26224	26224	26224	0	https://spati.ala.org.au/govseer/wms?service=WMS&version=1.1.0&request=GetMap&layers=ALA.Distributions&format=image/png&viewparams=6448
PROCELLARIDAE	5172	TRUE		urn:isid:biodi	7042129	FALSE	Dagapomoma	FALSE	TRUE	Dagapomoma australis	https://spati.urn.isid.biodi.urn.isid.biodiversity.org.au	Expert distrib d#804	10	2.8817E-05	G	Wilson's Storm-petrel	37 228	25154	154	25154	0	https://spati.ala.org.au/govseer/wms?service=WMS&version=1.1.0&request=GetMap&layers=ALA.Distributions&format=image/png&viewparams=5172
PROCELLARIDAE	5381	TRUE		urn:isid:biodi	7042427	FALSE	Fulmarus	FALSE	TRUE	Fulmarus glacialisoides	https://spati.urn.isid.biodi.urn.isid.biodiversity.org.au	Expert distrib d#804	2	2.8817E-05	G	Southern Fulmar	37 228	25157	25157	25157	0	https://spati.ala.org.au/govseer/wms?service=WMS&version=1.1.0&request=GetMap&layers=ALA.Distributions&format=image/png&viewparams=5381
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35032041	Pyura prae	Heller, 187 species		Animalia	Chordata	Ascidiacea	Stolidobrara	Pyuridae	Pyura	cunjevoi		
https://prc	(Miq.) Tieg	species	Plantae	Charophyt	Equisetops	Santalales	Loranthace	Amyema			Queensland : Conservation Status Northern Territory : Conservation Status	
urn:lsid:bic	Hydrurga lk	(Blainville, species		Animalia	Chordata	Mammalia	Carnivora	Phocidae	Hydrurga	leopard seal	South Australia : Conservation Status Queensland : Conservation Status	
urn:lsid:bic	Tursiops al	Charlton-R species		Animalia	Chordata	Mammalia	Cetacea	Delphinida	Tursiops	burrunan dolphin		
urn:lsid:bic	Scutiphora	(Kirby, 182 species		Animalia	Arthropod:	Insecta	Hemiptera	Scutellerid:	Scutiphora	Metallic Shield Bug		

Annex B HDD Technical Guide

Technical guide:

**information and advice for the successful planning
and execution of horizontal directional drilling works**



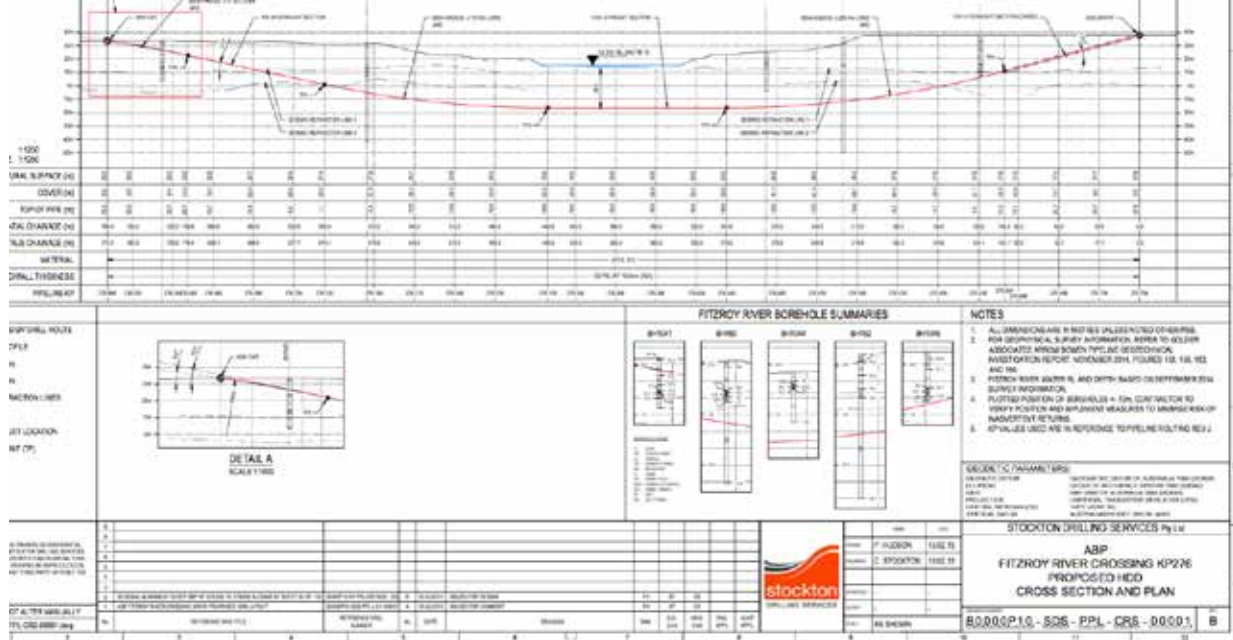
by Charles Stockton

Director and Senior Consultant

Stockton Drilling Services

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SO...WHAT EXACTLY IS HDD?

Horizontal Directional Drilling (HDD) is a trenchless method of installing pipeline or conduit and cables underground, along a predetermined path, by the use of specialised drilling equipment. This approach provides a flexible way of installing pipes and cables where conventional open cut methods are not permitted, practical, or environmentally or economically viable.

HDD is very useful in built up urban areas or where various obstacles or terrain, such as shore approaches, swamps and river crossings lie along the proposed route. By using HDD, the pipeline or cable land disturbance is only required at the entry and exit point and the pipeline or conduit is installed underneath the obstacle.

Trenchless technology allows for the installation or rehabilitation of a pipeline, conduit or casing between a given entry and exit point without any disturbance to the natural surface between those points. When new assets are installed using trenchless technologies, a tunnel is installed by directional drilling, microtunnelling, auger boring or impact moling. These different techniques allow for a great variety of distances, depths and diameters to be installed whilst minimising social and environmental impacts. HDD is usually employed when the required installation depth and length exceeds the boundaries of the other trenchless installation methods. HDD is most commonly used for road, river and shore crossings.

In order to take full advantage of the benefits offered by HDD and produce designs that can be efficiently executed in the field, design engineers should have demonstrated working knowledge of the process, including both industry capabilities and limitations.

The HDD process is usually undertaken in three distinct stages:

- » Pilot hole drilling
- » Reaming or hole opening
- » Pipe installation

The first stage consists of directionally drilling a small diameter pilot hole along the pre-determined pathway. In the second stage, the pilot hole is enlarged by successive reaming and cleaning passes. When the hole is of the suitable diameter and condition, the pipe is pulled back or thrust into the fluid filled bore.

KEY BENEFITS OF HDD

HDD has become increasingly versatile in its application and progressively more reliable in its execution. Today, the question is not “Why use HDD?” but rather “Why not use HDD?”.

The key benefits of employing HDD include:

- » Access for construction equipment is only required at either end of the crossing
- » Disturbance is only required at the entry and exit points and not for the entire crossing length
- » The limited work footprint required at either side of an installation allows it that it can cater for all weather working conditions
- » Due to a constrained work site, it can be easily adapted to a 24-hour program to safeguard the works schedule
- » Due to the static position of major plant, safe access and egress, the site can be developed for worker safety – no deep pits or shafts are required and all work is performed above ground
- » Alternative tooling and mud formulations allow for all soil conditions to be drilled
- » Unforeseen ground conditions can be quickly addressed by changing the tooling and drilling fluid formula with limited cost and schedule impact
- » HDD can be used along with other technologies such as retractable tunnelling and direct pipe to overcome instability problems, such as cobbles and cohesiveless soils
- » Flexibility in the profile design allows for engineers to develop creative solutions to difficult geometries, including compound bends and steep entry and exit angles, with the only limit being the minimum radius of the installed pipeline material

In order to take full advantage of the benefits offered and use HDD for increasingly difficult crossings, HDDs should be designed and planned by experienced personnel who have proven working knowledge of the process, including both industry capabilities and limitations. Designs should be constructible and be able to be efficiently executed in the field.



THE CURRENT LANDSCAPE

Since trenchless technologies such as HDD, were first introduced in Australia, the price of using these methodologies has become increasingly more affordable. At the same time, the expertise and reliability of HDD contractors has increased, and so the use of HDD in pipeline construction has become widespread.

In fact, today, very few pipelines are built that do not include directionally drilled crossings somewhere along the route. For example, the recent installation of an 180km pipeline in Victoria used HDD to complete over 80 crossings of roads, rivers, drains, sensitive habitats and difficult terrain.

For oil and gas projects, the design process is driven by a risk based approach, whereas for some utility projects, price tends to be the driving factor. Oil and gas projects proceed through a number of development phases that allow risks to be fully evaluated, both before and after tender. This ensures the design evaluation process is driven by best for project outcomes. Adopting this risk-based approach ensures, good safety, environmental or quality performance.

INTRODUCING CHARLES STOCKTON

UK-born Charles Stockton, Managing Director of Stockton Drilling Services, has been a part of the HDD sector in Australasia since 2003.

Charles graduated with civil engineering degree from Loughborough University in the UK before joining the family business Stockton Pipelines. Stockton Pipelines pioneered the development of HDD in the UK in the 1990s, with Charles managing the drilling side of the operations. Charles was part of the first HDD drilling crew in the UK, and in those early, pioneering days, he worked with crews to meticulously plan and execute HDD crossings across the country and northern hemisphere. They successfully completed a number of complex projects discovering and developing innovative solutions along the way, which are now part of normal HDD operations.

During his tenure at Stockton Pipelines, Charles became an industry leader, and received three drilling awards from the UK Society for Trenchless Technology for new technologies and projects of special interest.

Two years after arriving in Australia, Charles established Stockton Drilling Services, a premier engineering consultancy specialising in HDD and other trenchless pipeline installation methods, bringing his wealth of experience to the Australian market.

The company's client base includes companies in the oil and gas, water, electrical, mining, communications and pipeline engineering industries; local and state government departments; and HDD and pipeline contractors. Charles received the Engineers Australia Environmental Award for his part in Chevron's Gorgon Project, which required nine shore approaches to be constructed by HDD on the hostile west coast of Barrow Island in Western Australia.

Charles has a team of highly experienced project engineers, managers, supervisors and HSEQ personnel, who provide a range of services including concept evaluations, feasibility studies, engineering design, project management, quality control, risk management, HSE services and pipeline mapping.

Stockton Drilling Services personnel ensure the quality of planning and construction of their HDD projects by using a risk-based design approach, mentoring both clients and contractors on the use of industry recognised best practice.

In response to industry need, Charles developed a HDD quality control package that has already had excellent outcomes for ExxonMobil, QGC and APLNG. As part of the HDD quality control package, Stockton Drilling Services provide FEED engineering and quality control inspection services during construction.

Major projects that Charles has provided engineering services for include:

- ◆ Esso, Longford Liquids Pipeline
- ◆ Chevron, Gorgon Gas Project
- ◆ ExxonMobil, Longford Gas Conditioning
- ◆ QCLNG/APLNG, Narrows Crossings
- ◆ Arrow Energy, Curtis Island LNG and Bowen Pipeline Projects



WHAT ARE THE CURRENT CAPABILITIES OF HDD?

As an installation method, HDD has been used for more than thirty years now on projects around the world. The capabilities of HDD equipment have grown considerably over this time period.

In the industry today, there certainly appears to be a recognisable trend developing – longer drills. Nowadays, pipelines are being installed in lengths of up to (and even in excess of) 4.5km by HDD.

Advancements such as intersect technologies (which allow crossings to be drilled from both sides meeting in the middle), larger capacity drilling rigs and the availability of pipe thrusters to aid pipe insertion, have all allowed for much longer lengths to be regularly and reliably achieved. Of particular note is the use of intersect drills, which has effectively allowed drill lengths to be doubled overnight.

The industry is also able to competently install pipelines of significant diameters, with steel pipelines of up to 48-inches being regularly installed with HDD. It's important to note that end users rarely require diameters over this size to be installed. Once the diameter does become larger than 48 inches, the construction of a concrete tunnel is generally advocated as lower risk, and accepted as the norm.

It is reasonable to infer that lengths of 4.5km of 12-inch pipe and 2.5km of 42-inch pipe are now well within industry capability.

This capability will allow for greater flexibility in construction options, and clients should be incorporating this advancement into their concept designs and studies.

I believe the HDD industry will drill longer yet, aided by further developments such as:

- » Industry-specific software for analysing drill pipe pressures, fatigue and stress
- » Telescopic casing to provide hole support whilst reducing torque and drag
- » Mud programs and modelling for better hole cleaning and hole support, and
- » Equipment advancements including telescopic rigs for faster tripping times

HDD will also continue to adapt and expand into new markets as new demands emerge. For example, HDD can be used to place a permeable membrane in an exact underground position to allow for the introduction or removal of fluids. This process can then be used to control water flows, as was employed by Stockton Pipelines to control groundwater levels associated with the construction of the Cardiff Bay Barrage. A similar process may also be used for applications including mine dewatering and extraction of minerals rich sands.





ESSENTIAL FACTORS FOR A SUCCESSFUL HDD INSTALLATION

When undertaking a HDD installation, the key factors for consideration that often come to mind are geotechnical conditions, alignment geometry, installation constraints, and pipe strength rating. While these are all important, there are three other key factors that Clients and Contractors should take into account for a successful HDD installation.

D_O_N_'_T_'_T_A_K_E_'_S_H_O_R_T_'_C_U_T_S_'_'

After spending 25 plus years working on major HDD projects around the world, there is one golden rule that comes up time after time, and that is: don't take shortcuts. Do the right thing first time, every time.

If you speak to any experienced driller or superintendent who has worked on large-scale projects they will all tell you the same thing – don't take shortcuts.

Quite often site crews start to feel pressure from both the client and their own head office if they start to fall behind program.

Typically there is a reason that the schedule is slipping, and generally it is not inefficiencies or inexperience. It is more likely that the initial program was unrealistic or didn't correctly factor in some of the challenges or risks of the project, such as site constraints, geology or weather conditions, or complying with client requirements.

This is the time when crews will then be tempted to try and save time by initiating a shortcut that they know isn't good practice, but they think they can get away with. This is when small problems start to compound.

As most people have experienced with drilling, the stars very rarely align and you must make your own luck. Sticking to best practice and avoiding short cuts, even when all those around you are screaming for more progress, does this.

LEARN LESSONS

In larger companies with multiple crews, each driller or superintendent will tend to have their own way of doing things; their own favourite tooling configurations, preferred mud formula and site layouts.

Each supervisor will often be reluctant to heed the advice of other supervisors, until they too have learned the lesson firsthand.

This type of process can be hazardous and costly for the company (and the client). Even though each crew will be having these types of discussions on site about what is working for them in this particular condition and evaluating their performance, this valuable knowledge is rarely captured formally and is unlikely to be shared throughout the organisation.

If they are not doing this already, contractors should start to hold lessons learnt sessions at the end of each project to start developing their own rule book of best practice, that they can then rollout and employ throughout the company.

Only by continually evaluating performance can you start to achieve a professional outcome each and every time you go to site.

This feedback loop should also include head office to help them plan, create more accurate scheduling and develop more accurate pricing.

MAKE QUALITY DECISIONS

Quality decisions can only be made if a company encourages open dialogue throughout all levels of the organisation.

If the the Project Manager is dictating the course of action without first hearing and evaluating what others have to say, it will rarely be a good-quality decision. Supervisors should use reasoning, including the evaluation of facts and figures, over intuition.

To make a good-quality decision you should:

- » Define the problem clearly
- » Evaluate achievable alternatives
- » Collate meaningful reliable information
- » Determine required outcomes
- » Use logically correct reasoning to commit to a course of action



INTEGRITY, MAINTENANCE AND SAFETY IN HDD OPERATIONS


Integrity, maintenance and safety on HDD projects have come a long way, with more clients and contractors now aware of their importance on sites, and equipment designed to take these into account.

INTEGRITY AND MAINTENANCE

Due to this difficulty of accessibility and repair, a HDD section is usually designed with different parameters to a mainline, including being hydro-tested separately prior to insertion, and the use of increased coating thickness such as an abrasive resistant overcoat where deemed necessary. So not only is the HDD string pre-inspected, it is also then subject to a current drain test on completion of the pullback and prior to the tie-in to mainline to ensure the coating integrity of the HDD section. These additional quality checks, both before and after installation of steel pipelines, ensure the HDD section is unlikely to be the location of future integrity concerns.

If a defect exists which could potentially lead to a leak, the increased burial depth of the pipeline, which generally will be greater than 10m when installed by HDD, does provide added safety. Adversely the increased burial depth, along with the terrain being crossed – which may include a waterway or sensitive environment – makes dig up and repair prohibitive in most cases requiring the section to be replaced.

The coating system for the HDD's is required to be tolerant to coating damage during installation. Results of comparative testing have previously indicated that the coating thickness is directly related to the level of damage likely to be experienced during installation. For this reason, the preferred coating system for the HDD crossings should consider overall coating



thickness that can be applied. Where it is likely the coating will encounter sections of gravel, rock and sand during insertion, it is recommended the outer layer is considered as a sacrificial wear/abrasion coating with the base layer providing the anti corrosive protection.

Initially the HDD pipe section must be pulled over rollers and supports which create the over-bend for the pipe to enter the bore at the correct angle. For this phase of the works the coating should have good gouge resistance from potential contact with rollers and supports and also have flexibility to allow for the temporary over-bend radius to be formed.

In the next phase the pipe is pulled through the bore. A wide variety of geology may be encountered including alluvium consisting of sand, clay and gravel or bedrock, which should all be well defined in the site geological investigations. For sections in sand and rock the coating should have good wear resistance. For the sections in gravel the coating should have good gouge / shear protection and impact resistance as there's a potential the pipe will be pulled through unconsolidated sections which have partially collapsed.

In *Design and Coating Selection Considerations for Successful Completion of HDD Crossing* by A.I. Williamson and J.R. Jamerson, 3LPE performs well for abrasion, flexibility and impact but is ranked lower for gouge resistance. The 3LPE performed very well in the impact resistance test which is design to represent damage from falling rock. In the abrasion test the 3LPE also showed better performance than the other coatings. The polyethylene coatings appear to better resist wear due to the lubricity of the polyethylene particles compared to the harder nature of the particles from the other coatings.

It is difficult to say which of these tests (the gouge resistance, abrasion or impact resistance) best replicates the conditions downhole and it is likely to be a combination of all of these properties. Ideally the coating should be tough but not too hard where it has the potential to also become brittle.

SAFETY

Even though drilling crews are generally well established and familiar with all aspects of the work, management and supervision should continually use tools such as pre-start discussions, JHA's, SWMS and 5x5 to maintain crew focus on potential impacts from stored energy. These may be in the form of suspended loads, high pressure mud and hydraulic hoses, or rotating drill pipe and pipe tongs, to name a few. Crew should be trained to continually look for and identify these potential impact and then implement ways to prevent injury or damage.

HDD operations rely heavily on implementing safe working procedures and having sufficient experienced and trained personnel on site to manage the frequent lifting operations. During a pullback or during tripping drill pipe it may be necessary to lift pipe clear from the rig and stock piled them every two minutes for duration of several hours often in wet and slippery conditions. This process has the potential to become hazardous if clear systems of communication are not established and maintained.

HDD operations have the advantage that the sites are static and therefore allow for greater control of the work area. This allows for the site layout to be planned prior to mobilisation and include considerations such as a prepared hardstand; cables and hoses routes that can be buried or suspended; personnel access and egress that are clearly defined; include the use of bunds under static equipment; all of which allows for personnel to be familiar with their work environment.

The safety culture on HDD sites, like pipelines, has come a long way and all personnel now realise safety as a core value, not just a set of rules to be obeyed. Stockton Drilling Services is proud to have recently provided HDD Clients Representative for a pipeline project that installed over 18km and 70 HDD's with three maxi rigs and three mini rigs without incident.





DEALING WITH FRAC-OUTS

A ‘frac-out’ is the unintentional return of drilling fluids to the surface during horizontal directional drilling (HDD). A frac-out occurs when the down hole mud pressure exceeds the overburden pressure (i.e. shallow or loose sections of the bore), or the fluid finds a preferential seepage pathway (such as fault lines and fractures, infrastructure or loose material).

These fractures can be natural or induced by over-pressurising the formation. It is relatively common for a frac-out to occur on a HDD project. Most frac-outs, however, are usually minor, within construction right of way and close to the bore entry or exit.

VARYING LEVELS OF SERIOUSNESS

The seriousness of a frac-out depends on where it occurs. If the frac-out occurs in an environmentally or culturally sensitive area (which you are generally trying to avoid by using HDD), there is reason for concern.

The drilling fluid itself may not be toxic, but the fine particles can smother plants and animals, particularly in an aquatic environment. If a saltwater polymer fluid is used, the salt can also impact on freshwater systems and terrestrial vegetation. Neighbouring landowners do not appreciate frac-outs on their land.

In most states a frac-out outside of the working area is generally considered a “reportable incident”. In Victoria all frac-outs must be reported to the regulator within two hours.

Frac-outs may also damage infrastructure or nearby services. There are reports of sections of roads rising, nearby water pipelines failing as the frac-out washed away the bedding sand, power boxes filling with fluid and vegetation disappearing into a sinkhole caused by a frac-out.

On the other hand, the frac-out may be small (less than 20L), occur within a disturbed or non-sensitive area and be easily contained and cleaned up. In these cases, there is no lasting impact or damage and no real reason for concern.

These frac-outs are still better avoided as they utilise resources and time in the cleanup and reporting.

Generally with frac-outs, the perception and association with other industries means the perceived threat is far worse than reality.

PREVENTING FRAC-OUTS WITH GEOTECHNICAL INFORMATION

There are a number of steps that can be taken to prevent a frac-out from occurring.

The first step is to assess the risk of frac-out prior to drilling. This can be done using specially designed software (e.g. DGeo Pipeline by Deltares) or pressure calculations.

These methods compare the maximum allowable fluid pressure against the expected drilling fluid pressure.

To ensure they are reliable, they require detailed information on the soils, drilling fluids and bore profile, and should be conducted by experienced personnel.

The modelling will predict if and where frac-outs are likely to occur, if profile changes are required (e.g. increasing the depth), the maximum drilling pressures (the driller can then set alarms at these pressures) and if other management strategies are required.

In some cases it may be necessary to install casing at the entry point where reduced cover and bearing pressure exists, or drill pressure relief wells to give the fluid a controlled place to go.

The modelling also allows for the optimum pilot hole bottom hole assemblies to be configured for the formation, allowing the correct bit size to be selected for the drill pipe dimensions.

During drilling, contractors should continually monitor the drilling fluid properties i.e. mud weight, viscosity, gel strength, volume and pressure, to prevent frac-outs.

They can also include a pressure sub for real-time down hole pressure monitoring by the driller, allowing actual annular pressure readings to be obtained in real-time and then plotted against the modelled values.

If any unexpected variations or trends are observed then drilling should immediately cease and the cause investigated.

Common causes include a restricted or blocked annulus created by a buildup of cuttings which requires mechanical agitation and fluid flow to re-suspend and remove the blockage.

Contractors should be prepared with frac-out contingency plans and response equipment such as sand bags, vac-trucks and the like in place. Regular inspections should also be conducted along the drill path during pilot hole drilling.

Both contractors and client can take steps to prevent frac-outs, especially in sensitive areas, by undertaking adequate assessment and planning before drilling, and ensuring sufficient controls and monitoring are in place during drilling.





AVOIDING DAMAGE DURING HDD INSTALLATIONS

There are risks when undertaking a HDD installation, especially in urban areas where the risk of damaging underground and aboveground facilities greatly increases. However, there are several things that can be done to help mitigate risks and avoid damage to the installed pipeline.

The five most important things to think about are as follows:

PILOT HOLE PROFILE

The as drilled profile of the bore will have an effect on the pull force and abrasion the pipe is exposed to during pullback.

This may be in areas where doglegs (rapid change in direction) have been created, which often occur at formation changes from soft to hard or hard to soft, or where radii of the pipe have not been maintained.

The driller's log, the steering engineers log and survey data should be examined on completion of the pilot hole to identify any potential areas that could be out of specification or cause potential problems during reaming and insertion.

HOLE REAMING

The speed of the reaming pass should be calculated to ensure the correct pump volume has been used for the given penetration rate.

For example, the cut volume of a 24-inch ream following a 12-inch pilot hole is approximately 2m³. If the solids being removed are measured at 20 per cent of mud volume and pump rate is 1,000Lpm, then the ream should take ten minutes ($1,000 \times 0.2 \times 10 = 2,000 \text{ L}$) (2m³).

The driller's log should indicate the time per joint, and the mud logs/test report should indicate the percentage of solids in the mud returns. Also, as a rough guide, a volumetric check of the cuttings stockpiled on site can be equated against the complete hole volume.

MUD PROPERTIES

On completion of the pilot hole, and once the bore is open at both ends, the fluid must be configured to adequately suspend the cuttings indicated in the geotechnical investigations. The fluid viscosity and velocity must be tailored to create sufficient carrying capacity to facilitate the removal of the largest anticipated cutting size.

Cuttings suspension and transportation should be observed at the entry pit, and often cuttings will be deposited directly after exiting the bore.

This implies the fluid velocity, along with the viscosity (gel strength), is important in cuttings transport, but as soon as the velocity decreases after exiting the bore the cuttings fall out of suspension.

Mud logs and test records should be examined regularly to appreciate optimised fluid properties that were employed for each reaming stage, and their ability to suspend and remove coarse grained cuttings such as sand and gravel.

CLEANING PASS

It is good practice to conduct a cleaning pass with a under gauge barrel reamer after completing the reaming pass. For example, the barrel I would recommend for a 24-inch hole would be 20-22-inches. A smaller barrel would not correctly identify problem areas and potentially skip over or under any cuttings beds/restrictions/instability.

This pass should be used to gauge the condition of the bore and its readiness for pipe insertion.

Sometimes it will be observed that sections generate higher drill string torque, which would indicate cuttings, collapse or hole shrinkage. The driller should then swab back through the section to ensure hole stability before completing the pass. If any concerns remain, an additional cleaning pass can be re-run.

PULLBACK

The pipe must be correctly aligned with the borehole and enter the HDD central to the hole at the correct angle. A overbend plan should be developed to confirm the position and height of the supports and ensure that the pipe bend radius is maintained.

For large diameter steel pipelines and HDPE or FPVC pipes, the buoyancy of the pipe should be considered, as it displaces the drilling fluid from the bore. It may be necessary to fill or partially fill the pipe to create neutral buoyancy to reduce drag and therefore the insertion force and potential coating abrasion.



SHORE CROSSINGS WITH HDD

Working in the nearshore environment, especially on Australia's exposed coastlines, can be very challenging and inhospitable for both land-based and water-based construction equipment. Neither construction method is ideally suited to construction in the shallow, tidal, high-energy zone; it is too shallow for marine vessels, which risk grounding, and too deep and exposed for land-based work, which risks flooding and equipment damage. This is the zone that neither the offshore contractor or the onshore contractor are ideally placed to manage – it is not their normal working environment and it is this challenge that makes shore crossings very interesting to design and construct.

Up to 15 years ago, these crossings would have required large-scale open battered excavations onshore, which would connect to a piled cofferdam through the surf zone, followed by a dredged channel offshore. A concrete-coated pipe section would be floated into position using floatation devices and hydraulic winches. The process was very susceptible to adverse weather conditions and tidal variations, as well as presenting numerous challenges for managing worker safety and large-scale disturbance to the environment.

By using HDD to construct a shore crossing, you are totally eliminating the requirement for works to be constructed in the nearshore environment. Using HDD allows for the crossing length and depth to be increased, which positions the rig back on level land, and also allows for the exit to be beyond the surf zone. The rig can be placed well behind the dune system, preventing impact to the dune and any flora and fauna within the coastal corridor, e.g. shorebirds and turtle nesting areas.

The pipeline can be prefabricated onshore and thrust through the bore from entry to exit. Alternatively, the HDD rig can be used to pull back the pipeline from offshore if the pipe is fabricated by a laybarge or towed offshore from a spool base/launching area. This is especially convenient if the product pipe is HDPE, which will float without the use of external buoyancy control measures.

UNDERSTANDING THE CHALLENGES

Selecting an appropriate exit location. The exit point needs to be selected to provide appropriate conditions for positioning subsea structures or providing a suitable transition to the offshore pipeline. The exit location must provide sufficient water depth to allow safe vessel access and anchoring, as well as diving operations.

Obtaining reliable, cost-effective offshore geotechnical information. For shorter crossings it may be possible to interpolate onshore and nearshore boreholes, but for longer crossings it is extremely important to develop an understanding at the exit topography and geology. This is critical for developing the HDD methodology, determining whether the hole will be forward reamed or back reamed, and if the pipe will be thrust or pulled into the bore.

Weather and sea conditions. Even though the majority of the works can be conducted onshore, marine vessels and divers will still need to be deployed during a number of critical stages of the operation. Having these windows well identified, and then having contingency planning for delays, is essential. Clearly setting out what conditions the marine vessels can operate in, and determining how this risk will be costed will be important to prevent cost escalation and potential disputes between parties.

Specifying and managing a marine spread. A key difference with an onshore HDD operation is that the pipe-side will be managed over water by a marine spread. Marine activities may include seabed preparations, diving, lifting and recovery of downhole tooling, winching, tow-out, alignment and hook up of pipe string, placement of clump weights/mattresses for temporary stabilisation, as well as flooding and gauging of the pipeline. It is essential to correctly scope the marine work, specify the vessel requirements and establish the responsibilities and operational parameters. This is where using HDD contractors or consultants with previous experience working with marine operations is critical.

Discharge of drilling fluids at the exit point. Some shore crossings are undertaken in sensitive marine environments and discharge of drilling fluids and cuttings from the bore hole is not desirable. One technique that has evolved is to drill the pilot hole and leave the bore closed just prior to exit or plug it with an inflatable plug. This then allows for the bore to be opened by forward reaming, and drill fluids are returned to entry for recycling rather than being lost to the ocean floor. The final section of the bore can then be reamed out using biodegradable fluids to limit any potential environmental impacts of the break through to the seabed.

EXPERIENCE MATTERS

Stockton Drilling Services has been involved with a number of complex shore crossings constructed in Australia over the past 15 years, including:

- ◆ Minerva Shore Crossings (two) in Victoria for BHP Billiton
- ◆ Gorgon Shore Crossings (nine) in Western Australia for Chevron
- ◆ Kupe Shore Crossings in New Zealand for Technip/Origin Energy
- ◆ Victorian Desalination Pilot Plant Shore Crossings in Victoria for Department of Sustainability and Environment
- ◆ Narrows Shore Crossings (four) in Queensland for APLNG/QGC
- ◆ Gladstone Harbour Crossing Feasibility Study for Arrow Energy
- ◆ Anglesea Water Reclamation Plant (WRP) Shore Crossing in Victoria for Barwon Water

Two projects that are interesting to note are the Gorgon Shore Crossings and the Anglesea WRP Shore Crossing Replacement.

The Gorgon Shore Crossings were constructed on a Class A Nature Reserve, and won the national Environmental Engineering Excellence Award at the Australian Engineering Excellence Awards in Canberra. Ian Pedersen, Chair of the National Engineering Excellence Awards Judging Panel, said “the uncompromising environmental commitment to this project suggests engineering construction techniques can be ecologically sensitive, allowing us to maintain our natural environment for the future”.

The second project, which clearly indicates how the development of new construction techniques have allowed for improved design, is the Anglesea WRP Shore Crossing Replacement, located 25m above sea level on the Anglesea coastal cliffs. The previous outfall consisted of a 30m deep drop structure which transferred flows from the treatment plant level to the base of the cliffs. A 185m outfall pipe then discharged flows from the base of the drop structure to the ocean.

The outfall was constructed in 1995. Since construction, cliff erosion had exposed a section of the outfall pipe that runs through the base of the cliff, from the drop structure to the beach. The current rate of erosion is estimated at four meters every 10 years.

In May 2006, a rock fall crushed a section of the exposed pipe and emergency repairs were required. The pipe was repaired and a concrete block was formed around the exposed pipe. However, the cliff continued to erode further, exposing the pipe again, undermining the concrete

block and placing the pipe at risk of failure again. The instability of the cliff and risk of further collapse meant it was too dangerous to carry out temporary repairs to the broken pipe.

As a result Barwon Water initiated a project which required the design and construction of a new outfall pipeline and associated works. Stockton Drilling Services provided engineering support and construction supervision for the project.

A geotechnical desktop assessment was undertaken to allow for the design of alternative preliminary drilling profiles, and to define the scope for further geotechnical investigations. The project team then undertook bathymetric surveys and seabed sampling of the works corridor to establish suitable exit point locations. Seabed profile, water depth, currents, geology and environmental impacts were then evaluated to determine the lowest risk and optimised length and location for the drill exit and diffuser installation.

Considering the peak flows from the water reclamation plant and installation forces, it was determined that a 450mm diameter HDPE pipeline would be required. The pipeline would extend 700m from within the plant boundary to approximately 500m offshore to a water depth of 15m, where a 16m-long diffuser would be installed. The pipeline was installed within three weeks of mobilising to site.





UTILISING HDD IN URBAN ENVIRONMENTS

The execution of HDD in an urban environment is technically the same as for an open environment. The main difference is not what is happening below ground, but what is happening on the surface.

BENEFITS OF HDD IN DEVELOPED AREAS

The key benefits enjoyed by both contractors and residents are the reduced amount of open trench excavation, reduced time of construction and reduced amount of reinstatement. Open trenching can be difficult and time consuming in an urban environment due to the need to expose, protect and support existing services that cross the alignment. Often these trenches, curbs, pavements and roadside easements are then prone to future settlement after open trench installation methods have been used, which is eliminated by using HDD. The use of HDD may be essential if a main highway, railway or infrastructure needs to be crossed.

HDD is commonly used for pressurised water supply, pressure or graded wastewater, electrical conduits, telecommunications, and gas supply networks.

TAKING THINGS INTO CONSIDERATION

The crossing design must take into consideration the increased density of infrastructure both above and below ground, including: road, rail, foreign services, foundations, power poles, and overhead cables. Undertaking a dial before you dig (DBYD) enquiry and site inspections should be used to identify all services. These services should then be proven by potholing of ground penetrating radar (GPR) to confirm their exact position and depth.

The presence of infrastructure may also prevent the use of traditional magnetic or walk-over steering systems and alternative steering solutions such as gyroscopic tools may be required.

Space is constricted, a smaller footprint is utilised, and equipment needs to be configured to individual sites to allow for efficient and safe drilling operations. On the pipe side, it becomes increasingly difficult to string long lengths of pipe in one continuous length, so alternatives need to be evaluated. These may include performing tie-in welds during insertion, or using HDPE, which is more flexible than steel and easier to handle. Also, pipe trailers can be utilised for smaller diameter coiled pipes.

The execution of the works should then consider any potential impacts on adjacent residents and businesses, including: traffic management, noise, dirt on roads, dust and light emissions. In such an environment these factors will usually result in restrictions on operations which need to be considered, e.g. restricted working hours, noise control, heavy vehicle movement restrictions.

The risk and potential impact of a frac-out can also be increased in an urban environment. Previous construction activities and installation of other services can reduce the bearing capacity of the soil or introduce pathways for fluid migration.

These crossings require a thorough scoping and risk assessment process to ensure that all potential risks are identified and controls and contingencies are in place. The crossing design needs to allow for safe access, appropriate space for equipment setup, suitable pipe handling methodology, and a profile design that provides adequate separation to all infrastructure and services, both present and future.

A VERSATILE SOLUTION

Due to the vast array of drilling rigs available today, HDD can be used for all types of pipe installation in urban environments [Is this statement really true?]. Compact, silenced, powerful rigs with automated rod loading, built-in power generation and on-board pumps all simplify the drilling operation and reduce the required footprint.

One of the most challenging urban environments I have worked in was, Bangkok, where we had to design and install 28km of 30-inch high-pressure steel pipeline by HDD.

The plan our design team developed was to drill the section as 16 crossings, each between 400m and 1600m long. Due to the rapid development and expansion of the city limits, there was no space available for pipe stringing. Once the project was underway imaginative strategies had to be developed, including welding and stringing on top of shipping containers, and floating strings out by threading them along the canal network of khlongs.

One of the most challenging urban environments I have worked in was in one of the world's most congested cities, Bangkok, where there are over eight million residents. Even though there is hardly space to walk, never mind drive a rickshaw, we had to design 28km of 30-inch high-pressure steel pipeline to be installed by HDD.

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THE IMPORTANCE OF GEOTECHNICAL INFORMATION

Around 90 per cent of HDD work happens below ground, so thorough and well thought out site specific geotechnical investigations are essential for planning any HDD project. Obtaining sufficient and correct geotechnical information can make or break a HDD project. The importance of defining the crossing geology should never be underestimated. This will allow for the profile, downhole tooling, drilling fluids and schedule to be accurately developed and costed.

Initially, a site visit and geological review should be undertaken to provide a geological overview of the area, which should then be used to determine the site specific investigations.

The site investigation needs to obtain sufficient reliable information to permit the safe and economic design of installation and permanent works. The investigation should be designed to verify and expand upon any information previously collected. Site investigations for all drills should include site inspection/surface investigation, topographic survey, identification of existing services and a geotechnical assessment.

Where the client undertakes the investigations, the HDD contractor should assess the completeness of the information provided and ensure it provides sufficient information for planning and execution of the bore.

Where further information is required, the HDD contractor should provide a proposal to the client outlining the objectives, requirements, and budget costs for any additional works required.

CONSIDERING THE BASIC GUIDELINES

Some basic guidelines for planning the scope of the geotechnical investigations include:

The level of geotechnical investigation required is a function of the length of the bore and the anticipated complexity of the subsurface conditions. While typical spacing is at least every 150–250m along the bore alignment, a minimum of two geotechnical boreholes is required for each bore where the bore length is greater than 300m.

Boreholes should be located to track stratigraphy and to detect the geological sequence, structure, and areas of significant change. When results indicate other anomalies or highly varying strata, then additional boreholes may be required.

The boreholes should penetrate through an elevation at least 3-5m below the depth of the proposed HDD profile to provide information for HDD design modifications, pilot hole deviations and ensure any potential rock formations have been identified.

Boreholes should be offset perpendicularly from the HDD centreline where practical by 10m.

Investigations should describe the soils and rocks encountered and recover samples for laboratory testing. Where soils are encountered, in-situ standard penetrometer testing sampling should be undertaken at selected depth intervals within the borehole.

Where frac-out modelling is required, the geotechnical parameters required for undertaking the modelling (e.g. unit weight, shear strength, friction angle, cohesion and Youngs Modulus) should be determined during the geotechnical investigation.

The likelihood of soil/groundwater contamination or acid sulphate soils should be determined prior to undertaking any investigations. If contamination is suspected (i.e. near electrical transformers, fuel storage, petrol stations, industrial land), samples should be tested for likely contaminants in accordance with the relevant guidelines for contaminated sites.

Boreholes should be backfilled to minimise the possibility of drilling fluid migration along the borehole during subsequent HDD operations. The upper 1.5m of land-based boreholes should be backfilled with the surrounding soil. Below 1.5m, a backfill mixture containing cement grout and a bentonite product to promote expansion is recommended. Cuttings from the drilling operation may be incorporated into the backfill mixture if considered beneficial.

A geotechnical report addressing the sampling program, laboratory analysis (including strength testing and particle size distribution), interpretation of geotechnical engineering properties, bore logs and a profile of the subsurface conditions shall be produced. Reduced levels of borehole data shall be included on the HDD profile drawings.

Probably 90 per cent of the work is happening below ground, so thorough and well thought out site specific investigations are the most basic and essential requirement for planning any trenchless construction project.

All formations can be drilled reliably if the soil conditions have been properly defined and considered during the design phase.





WHAT IS HOLDING HDD BACK?


For our industry to continue to grow we need to keep our minds open to new technologies and possible hybrid solutions between open and trenchless construction, such as ploughing and direct lay. Our industries currently coexist, but may well merge further in the future as new methodologies and possibilities develop.

HOW CAN WE IMPROVE THE REPUTATION OF THE HDD INDUSTRY SO THAT IT IS NO LONGER CONSIDERED HIGH RISK?

As our urban centres continue to grow and our demand for resources needs to be met, there will be continued growth for the HDD industry as long as contractors ensure clients the methods and practices are well engineered and reliable. For example, the Australian market has been slow to take advantage of new technology such as gyro surveying, which is widely used in other countries.

One recent application demonstrates its value and the need for accurate and independent confirmation of pipeline as-builts installed by HDD. A civil engineering contractor was engaged to construct a major road underpass and was in the process of drilling in the concrete piles when they severed a 300mm diameter conduit containing a 132kVA cable. According to the as-built supplied, they were more than two metres away from the three HDDs. We mobilised the next day and were able to survey the two remaining conduits, which contained the other phases.

We confirmed the position of the two remaining conduits to +/-150mm along the entire length of the crossing, therefore allowing the works to confidently proceed. In this case, they were lucky no-one was hurt and just had a major power outage to deal with. As most pipe bundles are installed with a spare, I would like to see all HDD crossings re-surveyed with the gyro tool to ensure pipeline owners and service suppliers have accurate and reliable as-builts. This will simply become more and more critical as HDD installed infrastructure continues to grow with population density.



I also feel that we, as an industry, need to increase the quality, reliability and performance of HDD services in Australia.

Our industry continues to expand, at all levels, and there should be a means to standardise performance and ensure product delivery. At the moment there are no real codes of practice that exist that are applicable to current practices and capabilities of the market. Often foreign or out dated references are used in contract documents; but these are rarely applied or used in earnest.

To give pipeline owners, engineers and contractors a valid and recognised guideline would greatly enhance industry performance, reduce construction risk and ensure best practices are employed. It would help put Australian construction practices forefront on the world stage, and would provide the fabric for the development of HDD QA/QC. Every other part of pipeline construction is regulated but often the most challenging part of the works, the HDD, is left without independent inspection and verification.

Stockton Drilling Services is hoping to help facilitate the pipeline industry and trenchless industry in working together to develop a code of practice/ guidance note for the planning and execution of HDD projects in Australia.



THE FUTURE FOR HDD

HDD is now employed throughout the construction industry, and these days it's surprising not to see a small HDD rig at the side of the road anywhere a pipeline, cable or conduit is been installed in an urban environment.


It is no longer only for special sections, but an integral part of any pipeline routing and design.

There is a distinct split in the capabilities of HDD contractors; either operating mini/midi rigs and concentrating on small lengths and diameters; or operating maxi rigs (greater than 100 tonnes pullback capacity) for the installation of large diameter pipelines. These two very distinct fields mean that contractors are generally specialists in their own area but not necessarily in both. Therefore it is important that the right contractor, with the appropriate equipment and relative experience, is selected for any works.

Asset owners and builders must enhance the opportunities that trenchless construction offers if they are to stay competitive and embrace safer and more environmentally conscious pipeline construction. Whether you are evaluating a 50m section under a driveway or 4,000m section under an inhospitable coastline, HDD must be part of that evaluation.

HDD will become even more commonplace in the next five to ten years, and owners will need to engage specialist companies such as Stockton Drilling Services to ensure designs are optimised by recognised industry experts, who will ensure there is seamless alignment between the owner's requirements and the contractors capabilities.

In response to industry needs, Stockton Drilling Services has developed a HDD quality control package that has seen some excellent outcomes. HDD is the most technical and often the most challenging part of installing a



pipeline, however unlike the other components, there are no inspection and quality controls. The process is often left solely to the contractor or client superintendent, who may have limited HDD experience.

Depending on the project phase, we provide the following:

- » FEED engineering design – to ensure a technically feasible and efficient crossing is designed
- » Contractor proposal reviews – to ensure the contractor’s proposal is technically acceptable and adopts industry best practice
- » Quality control inspection during construction – to ensure contractor compliance and quality control

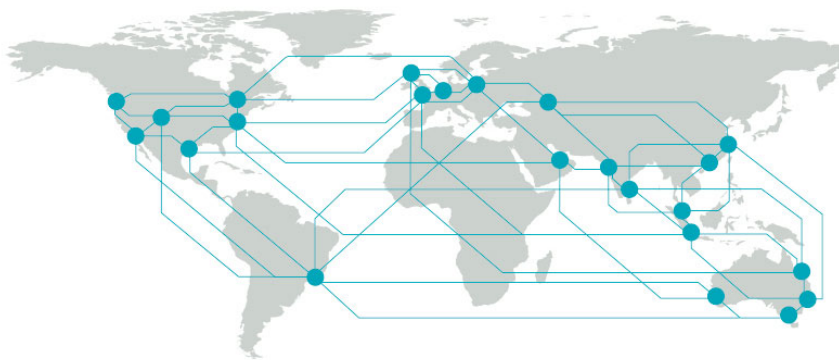
Our expert HDD inspectors are employed by clients in a site-based role to monitor and review the HDD construction process. The technical specialists monitor progress, identify issues before they arise, provide guidance, assist with problem solving, evaluate technical proposals, and monitor breakdown stoppages.

The benefits include:

- » Compliance with the scope, specification and industry best practice
- » Environmental impacts are avoided or minimised
- » Continual monitoring to ensure safe systems of work and equipment
- » Avoidance or minimisation of schedule delays and cost impacts by ensuring risks are identified early and managed appropriately

I feel that we, as an industry, need to continue to push the quality, reliability and performance of HDD services in Australia. Whilst our industry continues to expand at all levels Stockton Drilling Services continues to strive to standardise contractor performance and safeguard product delivery

Meanwhile we should all continue to explore new business opportunities that utilise HDD industry capabilities in new or unconventional ways including water management, mining applications and energy sectors such as geothermic. Horizontal Directional Drilling continues to be an exciting industry for clients, engineers and contractors alike and one that we are very proud to be support.



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Level 5
99 King Street
Melbourne
VIC 3000
Australia
+61 (0) 3 8620 6100

Registered in Australia
Registered no. 010 830 421
Registered office
Level 5, 348 Edward Street,
Brisbane QLD 4000 Australia

For your local BMT office visit www.bmt.org

Contact us

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